



**REVISED 2**

**UPDATE TO THE KICKING HORSE RIVER HYDRAULIC MODEL  
GOLDEN, BRITISH COLUMBIA**

Report Prepared for:  
**TOWN OF GOLDEN, BRITISH COLUMBIA**


Prepared by:  
**MATRIX SOLUTIONS INC.**

February 2014  
Calgary, Alberta

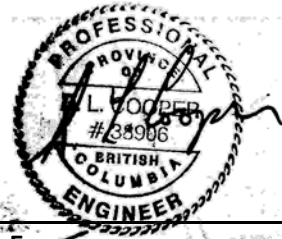
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Report prepared for the Town of Golden, February 2014

  
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**DISCLAIMER**

We certify that this report is accurate and complete and accords with the information available during the site investigation. Information obtained during the site investigation or provided by third parties is believed to be accurate but is not guaranteed. We have exercised reasonable skill, care and diligence in assessing the information obtained during the preparation of this report.

This report was prepared for The Town of Golden. The report may not be relied upon by any other person or entity without our written consent and that of The Town of Golden. Any uses of this report by a third party, or any reliance on decisions made based on it, are the responsibility of that party. We are not responsible for damages or injuries incurred by any third party, as a result of decisions made or actions taken based on this report.

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# 1 INTRODUCTION

## 1.1 Background

The Town of Golden is situated on the alluvial delta of the Kicking Horse River, as shown in Figure 1 and 2. The river exits from a narrow canyon at the upstream east side of the town and flows into the north flowing Columbia River at the downstream west side of the town. An armoured dike system along the Kicking Horse River combined with the existing road and rail embankments along the Columbia River act as flood protection for the town.

A free active channel on an alluvial delta tends to continually shift and cut new channels. During high flows it flattens out by building up and depositing material over its fan. The development of the town and dike system has restricted and modified this natural alluvial process on the river. However, deposition in the restricted channel portion remains an ongoing potential concern to future flooding. The growth of gravel bars in the lower reaches of the river reduces the flow capacity of the channel and decreases the level of protection provided by the dikes. Historically, the town has maintained the channel capacity by periodically removing gravel in the lower reach near its mouth with the Columbia River. After the 1997 excavation, due to potential aquatic impacts and concerns, justification for channel or bar excavation work was required. In 2003, a guideline justifying the need for bar excavation was recommended as follows: “where the cumulative average sedimentation rate in the lower reach from sections K6 to K55 is 0.3 m or more over April 1997 conditions” (Hydroconsult 2003). Alternatively, hydraulic modeling and environmental assessments are required to justify the need for excavation. Based upon this guideline, the bars were last excavated in November 2008 (Matrix 2012) after the amount of deposition was 0.43 m in 2007.

Several previous hydraulic model studies, repeated river cross-section surveys over time and sedimentation assessment reports have been conducted on the Kicking Horse River through the town. These studies have defined flood risk levels, prepared risk mapping, and assessed the effects of sedimentation and various levels of excavation. A summary of the reports and surveys completed are as follows:

1. Hydraulic modelling and flood risk assessments: initial mapping was in 1979 (BC Ministry of Environment 1979), dike assessments with no updated mapping completed in 1989, channel capacity assessments with the dykes and various levels of excavation and deposition (Hydroconsult 1999), updated flood risk mapping (Hydroconsult 2004).
2. Historic river cross-section surveys: initial surveys in 1975 with updates in 1987, 1997 (2), 1998, 1999, 2000, 2002 (2), 2005, 2006, 2007, 2008 (2), 2009, 2012, 2013. The years 1997 and 2008 were



surveyed two times for pre- and post-excavation. A second survey in 2002 was used for modelling purposes.

3. Sedimentation level assessment reports: nine annual review reports completed between 2000 to 2013, based upon the above historic surveys.

## 1.2 Objectives

In view of recent increased flooding concerns, the Town of Golden retained Matrix to update the hydraulic model on the Kicking Horse River at Golden to assess flood level risks based upon more intensive hydraulic cross-section surveys. The hydraulic model was initially completed using sections surveyed in 2012. Due to an above average flood peak in June 2013 the main hydraulic sections were re-surveyed in October 2013 and the model was updated again to assess any differences.

This study includes: calibration of the model based on previously applied roughness values and the June 2012 high water data, conducting the analysis for various selected return periods, and documenting updated freeboard levels along both left and right side dikes.

## 1.3 Study Basis

The analyses and assessment is based on the following:

- numerous site visits and site photographs by Matrix (and formerly Hydroconsult EN3 Services Limited (Hydroconsult)) from 1999 to present
- 2013 top of dike surveys, and detailed river cross-section surveys completed in April and October 2012 and October 2013 by Focus Surveys Ltd.
- historical Water Survey of Canada (WSC) streamflow data for the Kicking Horse River at Golden (Station 08NA006) and the Columbia River at Nicholson (Station 08NA002)
- historical air photographs from 1953, 1996 and 2008 and site river photos
- bridge plans and design drawings for the Highway 95 bridge provided by BC Ministry of Transportation and Highways, for the CPR bridge provided by Canadian Pacific Limited and for the pedestrian bridge at 8<sup>th</sup> Avenue North provided by the Town of Golden
- review of previous hydrologic and hydraulic study reports for the Town of Golden and region.

## 2 HYDROLOGIC REVIEW

The designated design flood in British Columbia for floodplain mapping and assessment is the 200-year peak discharge. The 1999 Hydroconsult report provided a hydrologic analysis of the peak flows based on historical records of the maximum daily discharges at two WSC stations near the study area:

4. 08NA002 Columbia River at Nicholson (10.2 km upstream of the confluence with the Kicking Horse River) with a drainage area of 6,660 km<sup>2</sup> and 93-years of record (1903 to 1997) at the time of the study.
5. 08NA006 Kicking Horse River at Golden with a drainage area of 1,850 km<sup>2</sup> and 34-years of record (1912 to 1922 and 1974 to 1997) at the time of the study.

Maximum mean daily flows for various return periods were determined using the Log Pearson Type III distribution frequency analysis. Maximum instantaneous flows were calculated by multiplying the maximum daily flow values by the average instantaneous/daily ratio for the three largest recorded flow events (ratios of 1.003 and 1.064 were calculated for the Columbia and Kicking Horse rivers, respectively). The maximum instantaneous 200-year peak discharge for the Kicking Horse River at Golden was determined to be 570 m<sup>3</sup>/s.

Previous estimates of the 200-year peak discharge of the Kicking Horse River at this location have ranged from 475 m<sup>3</sup>/s to over 750 m<sup>3</sup>/s. The 2004 report (Hydroconsult 2004) provides a detailed listing of previous site-specific and regional studies as well as a summary of the seven largest flood events from 1916 to 1999. Further hydrological calculations were completed as part of the 2004 report including a single station flood frequency analysis, a two station comparison frequency analysis and an analysis based on the runoff depth approach as developed by Alberta Transportation.

Based on the 2004 review, the 200-year design discharge value of 570 m<sup>3</sup>/s was conservatively recommended, as previously used in the 1999. This current update of the hydraulic model will also use this design discharge value, as the recent flood events from 2004 to 2012 have not significantly altered flood frequency analysis results. The 500-year peak discharge was calculated using a logarithmic extrapolation of the lower return period flood events. Table A lists the resulting flood flow frequencies.

**TABLE A Columbia River and Kicking Horse River Maximum Instantaneous Discharges**

Return Period (Years)	Columbia River upstream of the confluence of the Kicking Horse River (m <sup>3</sup> /s)	Kicking Horse River at Golden (m <sup>3</sup> /s)
2	428	245
5	528	306
10	586	351
20	638	397
50	698	461
100	741	514
200	777	570
500	861	632

Figure 3 plots the annual maximum instantaneous and daily discharges recorded at Station 08NA006 up to 2012. The period of record consists of 33 years of instantaneous discharges and 50 years of daily discharges. The highest reported flow was a daily value of 402 m<sup>3</sup>/s in 1916.

## **3 HYDRAULIC MODELLING**

### **3.1 2004 Study**

The current hydraulic analysis provides a comparison from Hydroconsult's 2004 report to assess any changes in the predicted 200-year flood profile and identify any potential impact of recent sedimentation. The 2004 study was based upon cross-section survey data up to November 2002. To compare with the 2004 study, annual sedimentation assessment reports based upon river cross-section surveys have tracked changes in the amount of sedimentation over time. The last formal review (Matrix Solutions 2012) presented a comparison of all historic surveyed sections up to April 2012. The historic surveys are therefore not repeated here. Since then the October 2012 and October 2013 section surveys were completed and an interim sedimentation review was conducted in August 2013 (email to the Town) based upon the October 2012 surveys. The extent of historic sedimentation based upon the section surveys is discussed later in Section 4.

### **3.2 Input Data**

Two river hydraulic river models were constructed for the same reach of the river as the 2004 study. Initially modelling was based upon April and October 2012 section surveys but after an above average flood peak in June 2013, the sections were re-surveyed in October 2013 with additional sections inserted to re-model the river.

#### **3.2.1 2012 Survey Model**

The first model was based on the most recent data for each section prior to the 2013 freshet. This included 38 sections surveyed in October 2012 (1+974 to 0+000), four sections surveyed in April 2012 (2+742 to 2+213, upstream of Highway 95 bridge) and five sections not previously surveyed that still relied upon the 1987 sections (3+583 to 3+132 and 2+184 to 2+068).

This survey captured significantly more detail between station 1+974 (K5 near the downstream end of Gould's Island) and 0+000 (the confluence with the Columbia River). Previous surveys only had 11 sections within this reach compared to 38 sections at an average spacing of 50 m with this current survey. The purpose of this was to document if the increased detail showed any significant differences in the 200-year flood profile.

### 3.2.2 2013 Survey Model

A peak discharge of 280 m<sup>3</sup>/s was recorded on the morning of June 21, 2013. This discharge corresponds to between a 2 and 5-year recurrence interval flood event. A survey was completed in October 2013 to capture any changes to the river cross-sections as a result of this event and to update the sections that had not been surveyed since 1987 (K1, K52, K2, K11b and K11). Additionally, three new sections (designated as Section K60, K61 and K62) were surveyed at key locations to provide a more accurate model of the river profile. As a result, the 2013 survey provided a more detailed refinement of the model from that used in 2004 but with less detail in the lower reach than the 2012 survey.

### 3.2.3 Top of Bank Survey and Model Parameters

A June 2013 survey of the top of banks along the Kicking Horse River was also included to re-assess the current freeboard levels along the dikes.

All other hydraulic model parameters (e.g., roughness, expansion/contraction coefficients, bridge data, starting downstream water level, and calibrations) used in the 2004 and 1999 studies were repeated in the 2012 and 2013 section models. This includes assuming coincident flood peaks on the Columbia and Kicking Horse rivers to define maximum backwater effects. Backwater effects from the Columbia River and the timing of the peaks were discussed in the 1999 report. Assuming coincident peaks with the Columbia River is not significant because it only affects the lower 1 km long reach of the Kicking Horse River. There is minimal impact from this assumption because of the limited development in this lower reach and the Columbia will backflood the other side of the dike to a comparable flood level. The input data used in the present analysis are provided in Appendix A.

The flows listed in Table A were all run in the hydraulic analysis.

## 3.3 Model Calibration

An annual maximum instantaneous discharge of 352 m<sup>3</sup>/s was recorded during the night of June 6, 2012. It was the third highest recorded discharge since 1981. The peak water level at this time was observed to be approximately 0.2 m below the low point in the right berm at Station 1+990 downstream of Highway 95 bridge (Figure 2). The estimated elevation of the right bank at this location is 787.24 m resulting in an estimated water level of 787.04 m. This is an approximate estimate that was not surveyed in at the time.

This peak discharge event is equivalent to the 10-year peak discharge based upon the flood frequency values in Table A (Section 2). The modelled 10-year water level at this location (using the same hydraulic model parameters as previously used) corresponds to a water level of 786.87 m in the 2012 model and 786.86 m in the 2013 model. Therefore the modelled water level is 0.17 to 0.18 m lower than the observed June 2012 peak water level. To increase the modelled water level by this amount would require increasing the originally calibrated Manning's n value at this section from 0.025 to 0.030.

This adjustment in the roughness coefficient would result in a localized increase in the 200-year water level of 0.20 m at the section immediately upstream (K11B) and only 0.03 m at the next three sections upstream of K11B (K11 to K4). Considering lower channel n values are typically expected at higher flows, no adjustment was considered warranted for the higher 200-year flood flow based upon this one approximate data point. Therefore, the originally calibrated values used in the 1999 and 2004 studies were retained for use in the current study.

## **3.4 Model Assessment Results and Comparison with 2004 Study**

### **3.4.1 Comparison between the 2004 and 2013 Studies**

Table B compares predicted 200-year flood levels between the 2004 study and the present study. This table comparison shows localized differences ranging from -0.33 to +0.53 m with averaged overall differences balancing out.

The most dramatic differences were observed at or near the sections that had not been re-surveyed since 1987. These sections include K1, K52, K11 and K11B.

The most significant increase is at Station K11 where the modeled 200-year water level has increased by 0.53 m over 2004. This reflects the use of actual 2013 surveys at this section rather than the 1987 section survey that was used in the 2004 study. This section is located just downstream of the Highway 95 bridge and is a localized effect in the model. Higher levels are predicted just downstream and upstream of the bridge and slightly lower levels are predicted further downstream at sections K11B and K5.

The addition of new section K60 shows a locally higher predicted water level here than in 2004 (see the profile plot in Figure 4) reflecting the greater detail now in the model at this island.

The most significant water level decreases in modeled 200-year water level are observed at the downstream end of the reach (K53-K9) with an average decrease of approximately 0.17 m. This range of variation from plus or minus 0.2 m is expected due to natural bed level changes from year to year.

Overall, the more significant changes are more a reflection of refinement of the model at the islands and bridges and show the range of variation that is typical with the HEC-RAS model. This supports applying a 0.6 m minimum freeboard recommendation for the dike level to account for this level of model variation.

**TABLE B Kicking Horse River Hydraulic Sections Compared With Previous Hydraulic Study Results**

Section Name	Stationing (m)	200-year Water Surface Elev (m)			Difference between April/October 2012 and Present Study (m)	Difference between 2004 Study and Present Study (m)
		2004 Study	April /October 2012 Survey	Present Study		
K1	3583	794.74	794.80	794.65	-0.15	-0.09
K52	3519	794.85	794.93	794.53	-0.40	-0.32
K2	3132	792.67	792.57	792.64	0.07	-0.03
K50	2742	790.67	791.07	790.52	-0.55	-0.15
K60	2571	NS	NS	790.23	NS	
K51	2443	788.98	789.01	789.14	0.13	0.16
K4	2312	788.80	788.89	788.93	0.04	0.13
K10	2213	788.50	788.84	788.80	-0.04	0.30
Highway 95 Bridge						
K11	2184	788.13	788.47	788.66	0.19	0.53
K11B	2068	788.14	788.45	787.81	-0.64	-0.33
K5	1971	787.80	788.1	787.66	-0.44	-0.14
K61	1679	NS	NS	786.79	NS	
K6	1483	785.75	786.15	786.00	-0.15	0.25
K6A	1239	785.27	785.67	785.34	-0.33	0.07
K7	1106	785.04	785.37	785.15	-0.22	0.11
K7A	1015	784.87	785.16	784.84	-0.32	-0.03
K62	900	NS	NS	784.75	NS	
K7B	772	784.65	784.90	784.63	-0.27	-0.02
K8	643	784.49	784.75	784.48	-0.27	-0.01
K53	464	784.13	784.36	783.96	-0.40	-0.17
K54	410	784.22	784.41	784.06	-0.35	-0.16
CPR Bridge						
K9	275	784.16	784.12	783.98	-0.14	-0.18
MEAN					-0.22	0.00

Note: NS = Not surveyed for 2004 or 2012 studies

### 3.4.2 Comparison between the 2012 and 2013 Sections

Table B also provides a comparison between the 2012 model and section data versus the 2013 section data. As described in Section 3.2.1, the 2012 model was constructed using a combination of surveys from 2012 and 1987 but with detailed sections in the lower reach downstream of Gould’s Island.

This comparison shows a lower overall predicted 200-year flood level profile in 2013 compared to 2012 with local section differences ranging from -0.64 to +0.19 m. Again, the largest differences were observed at or near sections that had not been surveyed since 1987 (-0.64 m at Section K11B and -0.55 m at Section K50).

As with the comparison to the 2004 study, the downstream end of the reach (K7A to K9) predicts more significant decreases in the profile in 2013. The average decrease through this area was nearly 0.30 m from 2012 to 2013. This difference is primarily attributed to the increased detail in the number of sections used in the 2012 survey but is also somewhat due to local sediment flushing that occurred during the 2013 high flow event.

### 3.4.3 Available Freeboard

Table C lists all of the sections used in the model as well as the resulting water surface elevations for all of the modeled flow scenarios.

The left and right side dike freeboard is assessed here based upon the recent June 2013 detailed top of dike surveys and interpolating with the 2013 model 200-year water level results. The minimum freeboard recommended by the Ministry of Water Land and Air Protection of British Columbia is 0.6 m. Table C indicates the surveyed sections in red that have a freeboard less than this recommended minimum. Figure 5 provides a plot of the resulting freeboard in greater detail along the entire dike system compared with the 0.6 m recommended minimum freeboard. Figure 6 provides a layout and detailed list of all the reaches on each dike side where the freeboard is less than 0.6 m. The table in this figure could be used for designing minimum fill requirements. Section K9 (Station 0+275) is located just upstream of the confluence with the Columbia River and downstream of the town and is therefore not included in the freeboard analysis presented on Figure 6.

Due to the variations in potential sediment deposition (discussed in Section 4) and flood levels and ice jam risks, Matrix has previously recommended increasing the freeboard to 1 m as a medium to long-term target. As this may not be practical throughout the entire length of both dikes, higher priority reaches where the minimum 1 m freeboard target should be considered are indicated in Figure 7. These priority sections are all along the left side except for the low 210 m long right side section downstream of Highway 95 bridge where a long term solution will be required. The left side dike sections are as follows:

- upstream near the campground below Section K52 for 175 m
- along the island beside College of The Rockies at Section K50 for 180 m
- about 50 to 170 m upstream of Highway 95 bridge for 120 m
- downstream of Highway 95 bridge to the pedestrian bridge for 335 m

**TABLE C Kicking Horse River Hydraulic Analysis Results - 2013 Model**

Section	Stationing	Date of Survey	Thalweg	Right Top of Dike Elevation	Left Top of Dike Elevation	Flood Water Levels (m)							200--year Flood Freeboard (m)		
						2--year Flood	5--year Flood	10--year Flood	20--year Flood	50--year Flood	100--year Flood	200--year Flood	500--year Flood	Right Bank	Left Bank
K1	3+583	Oct--13	790.04	796.40	795.30	793.65	793.87	794.02	794.16	794.35	794.5	794.65	794.82	1.75	0.65
K52	3+519	Oct--13	791.25	796.64	795.30	793.38	793.61	793.77	793.93	794.14	794.33	794.53	794.78	2.11	0.77
K2	3+132	Oct--13	789.54	795.21	793.69	791.79	792.05	792.21	792.35	792.51	792.59	792.64	792.67	2.57	1.05
K50	2+742	Oct--13	787.63	792.30	791.60	789.8	789.88	789.94	790.02	790.22	790.36	790.52	790.72	1.78	1.08
K60	2+571	Oct--13	786.48	792.00	790.93	788.82	789.11	789.31	789.51	789.79	790.01	790.23	790.48	1.77	0.70
K51	2+443	Oct--13	785.34	789.85	790.46	787.99	788.25	788.42	788.59	788.8	788.97	789.14	789.34	0.71	1.32
K4	2+312	Oct--13	785.28	789.80	789.81	787.57	787.89	788.1	788.29	788.54	788.74	788.93	789.18	0.87	0.88
K10	2+213	Oct--13	784.25	790.32	790.24	787.34	787.66	787.88	788.09	788.36	788.59	788.8	789.07	1.52	1.44
Highway 95 Bridge															
K11	2+184	Oct--13	784.02	789.50	789.50	787.31	787.61	787.82	788.02	788.27	788.47	788.66	788.86	0.84	0.84
K11B	2+068	Oct--13	784.09	788.18	788.79	786.55	786.83	787.01	787.19	787.43	787.62	787.81	788.03	0.37	0.98
K5	1+971	Oct--13	783.78	788.41	788.50	786.33	786.63	786.82	787.01	787.26	787.46	787.66	787.88	0.75	0.84
K61	1+679	Oct--13	783.28	787.94	787.35	785.66	785.89	786.05	786.21	786.43	786.61	786.79	786.99	1.15	0.56
K6	1+483	Oct--13	783.17	787.78	787.80	785.26	785.44	785.55	785.66	785.8	785.9	786	786.09	1.78	1.80
K6A	1+239	Oct--13	782.15	786.77	786.33	784.51	784.68	784.8	784.92	785.08	785.21	785.34	785.49	1.43	0.99
K7	1+106	Oct--13	781.09	786.63	786.75	784.22	784.41	784.55	784.68	784.86	785	785.15	785.32	1.48	1.60
K7A	1+015	Oct--13	780.86	785.87	786.03	783.99	784.16	784.27	784.4	784.56	784.7	784.84	785.02	1.03	1.19
K62	0+900	Oct--13	780.79	785.76	785.22	783.87	784.03	784.15	784.27	784.45	784.6	784.75	784.94	1.01	0.47
K7B	0+772	Oct--13	780.57	785.31	784.97	783.57	783.78	783.92	784.08	784.29	784.46	784.63	784.85	0.68	0.34
K8	0+643	Oct--13	780.51	785.25	785.09	783.36	783.58	783.74	783.91	784.13	784.31	784.48	784.72	0.77	0.61
K53	0+464	Oct--13	780.55	784.58	786.87	782.94	783.12	783.25	783.41	783.63	783.8	783.96	784.24	0.62	2.91
K54	0+410	Oct--13	780.29	784.79	786.87	782.95	783.14	783.29	783.47	783.7	783.89	784.06	784.35	0.73	2.81
CPR Bridge															
K9	0+275	Oct--13	780.61	784.18	786.90	782.28	782.77	783.05	783.29	783.57	783.78	783.98	784.29	0.20	2.92



## 4 SEDIMENTATION RATES AND EFFECTS

Since 1997, Matrix (and formerly, Hydroconsult) has completed numerous assessments detailing the deposition (or scour) at repeated cross-sections at the downstream end of the reach, primarily downstream from Section K6. These assessments are intended to monitor sedimentation rates and assess if excavation or bar lowering is needed.

Historically, deposition has been identified as greatest between 0+643 and 0+899 (Sections K8 to K62). The current model however indicates that these sections have not significantly increased the 200-year flood level over the 2004 study. Previous reviews of sedimentation based upon sections up to October 2013 using areas and conveyance capacities only, without detailed hydraulic modeling, provides results similar to the hydraulic model (typically within  $\pm 0.1$  m). Therefore, this form of annual review without detailed modeling is considered adequate to assess sedimentation effects over time.

The detailed section surveys in 2012 show that the predicted 200-year flood level from upstream of the CP Rail bridge to upstream of Highway 95 averages 0.295 m higher than the 2004 study results. By comparison, the average depth of deposition from the surveys to 2012 was comparable at 0.275 m. A detailed topographic survey of the existing gravel bars was completed in October 2013. When compared to the October 2012 survey, the volume of material deposited up on the bars was only 543 m<sup>3</sup> greater in 2013 and the area of the bars had decreased by about 5%, or over 1,000 m<sup>2</sup> at similar elevations from 2012 to 2013. Due to channel bed scour in 2013, the net amount of deposition decreased from 2012 to 2013 by 0.14 m such that the current cumulative level of deposition is now at +0.13 m compared to the 1997 reference level. This is indicated in the historical record in Figure 8. This is now adequately below the 0.30 m guideline originally proposed to trigger the need for gravel bar lowering.

## 5 CONCLUSIONS

Hydraulic modeling of the Kicking Horse River based on surveyed sections in 2012 and October 2013 show the effect of sedimentation on flood levels and the available freeboard. Overall there have been minimal changes in the predicted 200-year flood profile since the 2004 study to the 2013 surveys. The greatest differences are localized where new sections were added or where old sections have been updated.

The comparison between the detailed section surveys in 2012 and the 2013 models show a greater net decrease (0.18 m) in the flood profile than the comparison to the 2004 study. It is expected that this net decrease has been skewed due to the inclusion of the 1987 sections in the 2012 model. Due to the high flows in June 2013, it is not possible to quantitatively measure the impact of the greater intensity of surveyed sections at the downstream end of the reach. However, due to the relatively minor differences

in the flood profile, it is expected that the sections surveyed in October 2013 are sufficient to provide an accurate representation of the current state of the river.

Applying the provincial guidelines, the dike system has adequate freeboard (at least 0.6 m) in almost all locations above the designated 200-year flood level equivalent to a peak discharge of 570 m<sup>3</sup>/s. Although the provincial guidelines specify the design flow for flood protection is the 200-year flow, the 500-year flow was also included in this analysis. The results show that the flow is still confined within the channel during this flood event throughout the majority of the reach. Previously assumed extreme case scenario evaluations show that dike overtopping can occur, likely as a result of a combination of extreme conditions (local sediment deposition, debris and increased values in assumed channel roughness as well as a flood peak exceeding 570 m<sup>3</sup>/s).

## 6 RECOMMENDATIONS

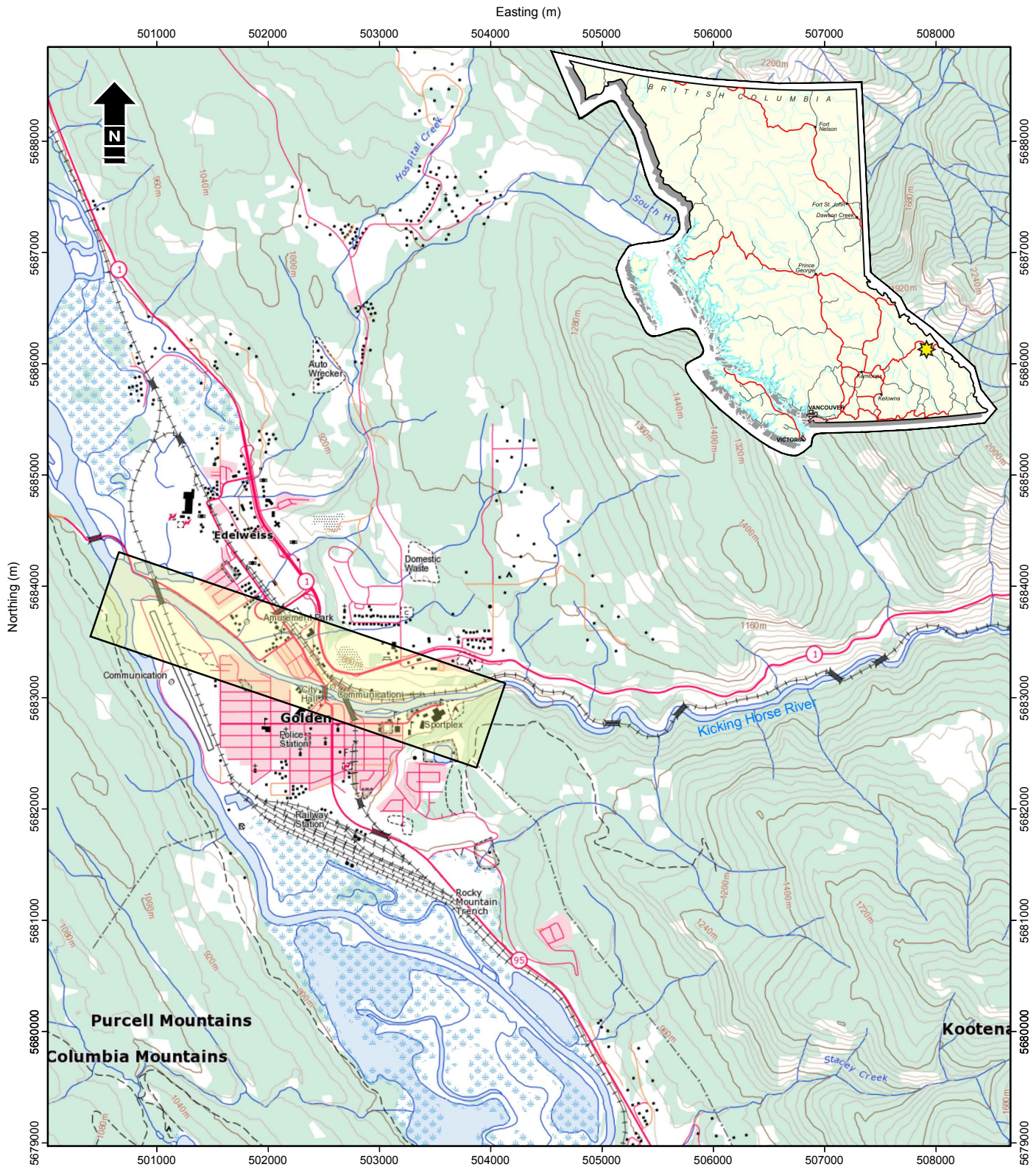
The following recommendations are provided based upon these study results and the previous Matrix studies conducted on the Kicking Horse River in Golden:

1. Continue conducting and evaluating river cross-section surveys at least every two or three years in order to continue to monitor sedimentation conditions as an ongoing program. However, as the level of deposition is midway between zero and the 0.3 m guideline target for justification of bar lowering, depending upon high flow conditions next year, the need for surveys should be evaluated in the lower reach (K6 to K55) in the fall of 2014.
2. The sections surveyed in October 2013 and illustrated on the figures are considered to be sufficient for hydraulic modeling purposes. Further detailed section surveys (at 50 m spacing) are not considered warranted at this time.
3. Figure 6 illustrates the areas of both dikes where the existing freeboard is less than 0.6 m. Adding fill to these locations to achieve the minimum 0.6 m freeboard is recommended, as a minimum. Wherever practical, increasing the freeboard to 1 m is recommended. High priority dike sections to achieve this 1 m freeboard and the corresponding fill levels are provided in Figure 7.
4. Continue the ongoing dike monitoring and maintenance program consisting of: annually inspecting the dike side slopes, the protective riprap and the dike crest, and replacing /stabilizing the riprap, as required.
5. Continue to update and test the Emergency Preparedness Plan that is in place on a regular basis.

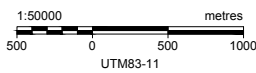
## 7 REFERENCES

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- Hydroconsult EN3 Services Ltd. 2006. *Update Assessment of Sedimentation on the Kicking Horse River, Town of Golden*. Report prepared for the Town of Golden, January 2006.
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- Hydroconsult EN3 Services Ltd. 2003. *Assessment of Sedimentation on the Kicking Horse River, Town of Golden*. Report prepared for the Town of Golden, February 2003.
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- Matrix Solutions Inc. (Matrix). 2010b. *Bridge to Bridge Dike Improvement and Riverfront Enhancement Plus Kayak Park - Hydrotechnical Assessment*. Letter report prepared for Town of Golden. Calgary, Alberta. October 15, 2010.
- Matrix Solutions Inc. 2010c. *Bridge to Bridge Dike Improvement and Riverfront Enhancement - Hydrotechnical Assessment*. Town of Golden. Golden, B.C. September 2010
- Matrix Solutions Inc. 2008. *2007 Update Assessment of Sedimentation on the Kicking Horse River, Town of Golden*. Report prepared for the Town of Golden, February 2008..
- Matrix Solutions Inc. 2006a. *Update Assessment of Sedimentation on the Kicking Horse River, Town of Golden*. Report prepared for the Town of Golden, 2005 Surveys. February 2006.
- Matrix Solutions Inc. 2006b. *Update Assessment of Sedimentation on the Kicking Horse River, Town of Golden*. Report prepared for the Town of Golden, 2006 Surveys. December 2006.
- US Army Corps of Engineers, 2002. *HEC-RAS River Analysis System Version 3.1*. Hydrologic Engineering Centre. Davis, CA., November 2002





Site Location



Town of Golden  
Flood Risk Assessment Mapping

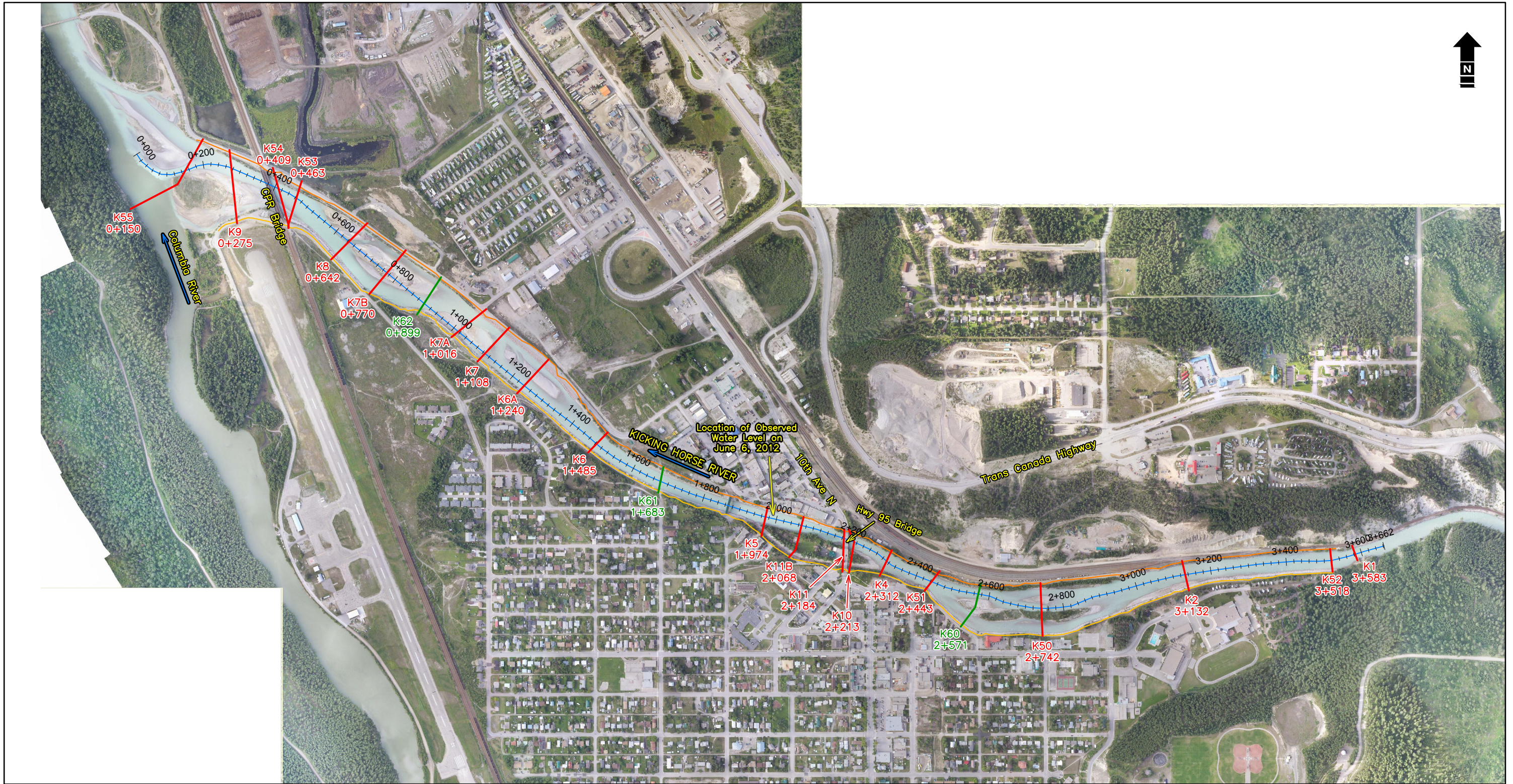
### Site Location Map

REFERENCE: 82N07 (Golden), Edition 4, UTM Zone 11, NAD 83  
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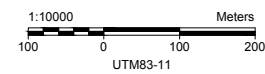
Date:	February 2014	Project:	5635-LP-14	Technical:	Z. Steele	Reviewer:	D. Cooper	Drawn:	C. Zhang
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- New Section - Not Used in 2004 Study - Used in Present Study - Surveyed in October 2013
- Section Used in 2004 Study - Re-Surveyed in October 2013
- Surveyed Right Bank - June 2013
- Surveyed Left Bank - June 2013



Reference:

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No.	DATE	DESCRIPTION	BY
4	2014/02/10	Revised Final	DLC
3	2014/01/23	Re- Issued Final	DLC
2	2014/01/06	Re- Issued Final	DLC
1	2013/11/13	Final	JB
0	2013/08/13	Issued for Client Review	DLC



Town of Golden  
Flood Risk Assessment Mapping

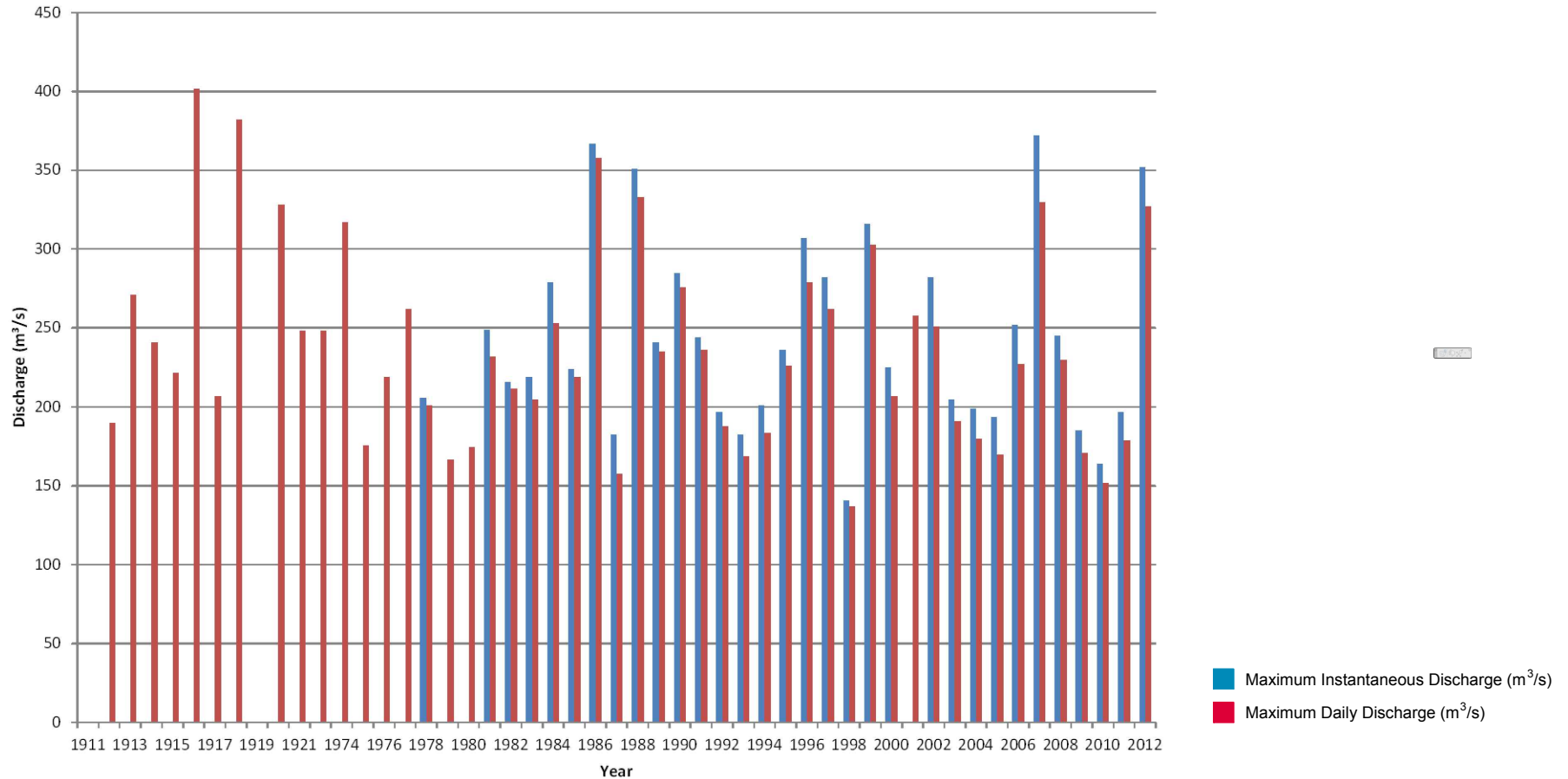
### Kicking Horse River Study Reach

Date: February 2014 Project: 5635-SP-C3D-14 Technical: J. Bigelow Reviewer: D. Cooper Drawn: Z. Steele

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### Annual Maximum Kicking Horse River Historic Flows



Reference:

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		4	2014/02/10	Revised Final	DLC
		3	2014/01/23	Re- Issued Final	DLC
		2	2014/01/06	Re- Issued Final	DLC
1	2013/11/13	Final	JB		
0	2013/08/13	Issued for Client Review	DLC		
No.	DATE	DESCRIPTION	BY		



Town of Golden  
Flood Risk Assessment Mapping

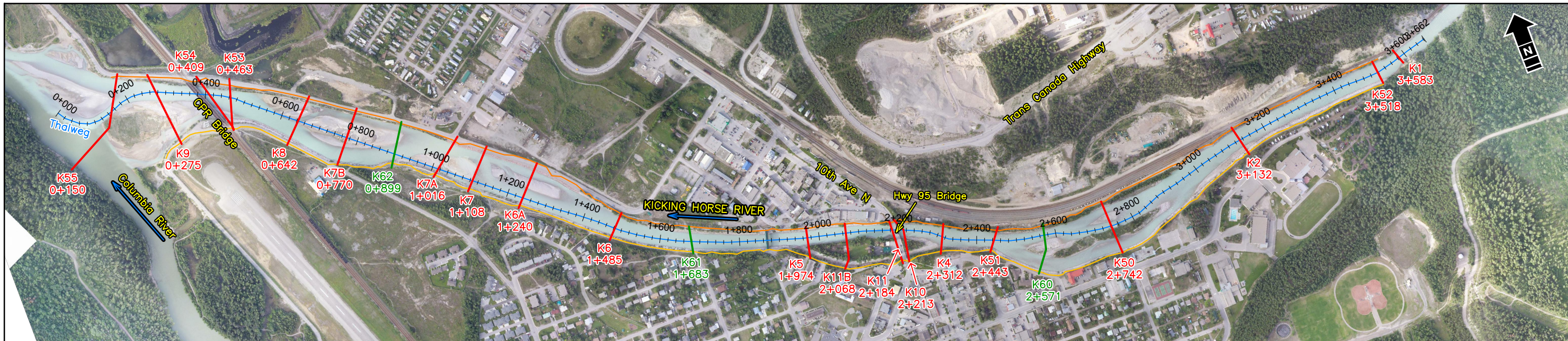
### Annual Maximum Kicking Horse River Historic Flows

Date: February 2014 Project: 5635-SP-C3D-14 Technical: J. Bigelow Reviewer: D. Cooper Drawn: Z. Steele

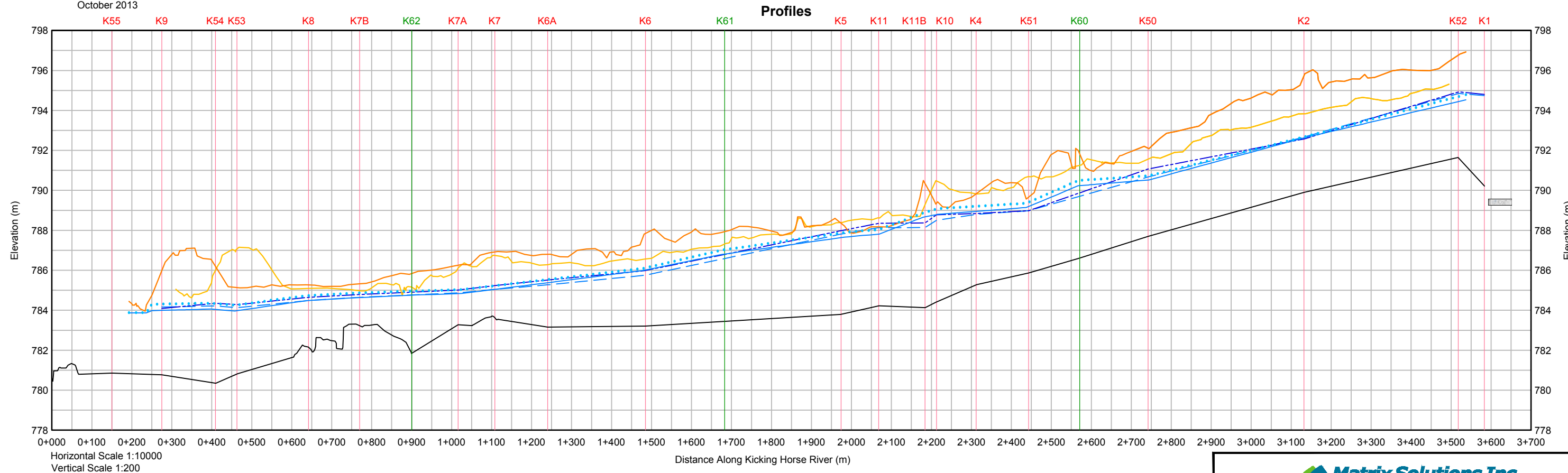
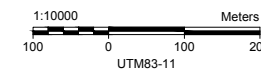
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Figure 3



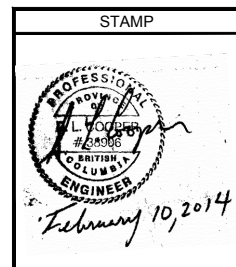


— New Section - Not Used in 2004 Study - Used in Present Study - Surveyed in October 2013  
— Surveyed Right Bank - June 2013  
— Surveyed Left Bank - June 2013  
— Section Used in 2004 Study - Re-Surveyed in October 2013



— Thalweg - October 2013  
— 1:200 Year Flood Level - 2013 Study  
- - - 1:200 Year Flood Level - 2004 Study  
. . . . . 1:500 Year Flood Level - 2013 Study  
- . - . - . 1:200 Year Flood Level - 2012 Study

Reference:



STAMP	PERMIT	REVISION	
		No.	DATE
		4	2014/02/10
		3	2014/01/23
		2	2014/01/06
		1	2013/11/13
		0	2013/08/13
		BY	

**Matrix Solutions Inc.**  
ENVIRONMENT & ENGINEERING

Town of Golden  
Flood Risk Assessment Mapping

**Bank Levels Versus Flood Discharge Levels**

Date: February 2014	Project: 5635-SP-C3D-14	Technical: J. Bigelow	Reviewer: D. Cooper	Drawn: Z. Steele
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**Figure 4**







Plot 111 = Tabbed (L) P:\9835\Drawings\2014\9835-SP-C3D-14.dwg - D:\K\JH - Monday, February 10, 2014 2:39:02 PM - Gary Evenson



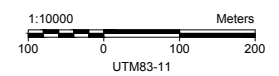
River Station (m)	UTM 11N (m)		Design Grade (m)	Fill Required (m)
	Easting	Northing		
1815	502161	5683194	787.79	0.00
1820			787.80	0.06
1828			787.82	0.07
1843			787.87	0.02
1853	502196	5683179	787.89	0.00
1959	502297	5683147	788.19	0.00
1982			788.26	0.05
1995			788.28	0.40
2005			788.30	0.45
2027			788.34	0.38
2048			788.37	0.20
2070			788.41	0.23
2090			788.57	0.41
2130			788.88	0.43
2148			789.01	0.48
2160			789.11	0.25
2171	502501	5683089	789.19	0.00
2203	502534	5683083	789.35	0.00
2211			789.38	0.07
2215			789.40	0.00
2228			789.42	0.25
2244			789.44	0.29
2260			789.47	0.04
2280	502604	5683044	789.50	0.00
2428	502733	5682982	789.73	0.00
2437			789.74	0.18
2457			789.91	0.02
2473	502776	5682965	790.04	0.00

River Station (m)	UTM 11N (m)		Design Grade (m)	Fill Required (m)
	Easting	Northing		
643	501167	5683796	785.08	0.00
666			785.11	0.00
689			785.13	0.02
711			785.16	0.08
733			785.18	0.13
750			785.20	0.17
762			785.21	0.22
775			785.23	0.26
785			785.24	0.27
796			785.25	0.19
815			785.26	0.00
836			785.28	0.00
850			785.29	0.01
854			785.30	0.03
860			785.30	0.00
869			785.31	0.05
876			785.32	0.18
881			785.32	0.18
885			785.32	0.16
886			785.32	0.18
891			785.33	0.14
900			785.33	0.17
911			785.34	0.16
912			785.35	0.11
918			785.35	0.25
933	501419	5683647	785.36	0.00

River Station (m)	UTM 11N (m)		Design Grade (m)	Fill Required (m)
	Easting	Northing		
1334	501728	5683390	786.19	0.00
1356			786.25	0.03
1376	501759	5683361	786.31	0.00
1440	501806	5683321	786.48	0.00
1461			786.54	0.05
1482			786.60	0.04
1500			786.67	0.07
1515	501867	5683273	786.73	0.00
1563	501909	5683246	786.91	0.00
1580			786.98	0.07
1599			787.05	0.00
1619			787.13	0.01
1641			787.21	0.08
1656			787.27	0.08
1672			787.33	0.12
1686			787.39	0.04
1694			787.41	0.05
1702	502036	5683180	787.44	0.00
1807	502137	5683146	787.75	0.00
1828			787.81	0.05
1850			787.88	0.06
1859	502186	5683127	787.91	0.00
2102	502407	5683003	788.59	0.00
2129			788.79	0.03
2155			788.99	0.31
2172			789.13	0.42
2189	502485	5682982	789.26	0.00

- New Section - Not Used in 2004 Study - Used in Present Study - Surveyed in October 2013
- Section Used in 2004 Study - Re-Surveyed in October 2013
- Surveyed Right Bank - June 2013
- Surveyed Left Bank - June 2013
- | Bank Low Areas

Note: The areas denoted on this figure indicate areas where the freeboard above the 200 year flood event is less than 0.6 m. The tables provide the fill required for each area to achieve 0.6 m freeboard.



Reference:

STAMP	PERMIT

REVISION		
No.	DATE	DESCRIPTION
4	2014/02/10	Revised Final
3	2014/01/23	Re- Issued Final
2	2014/01/06	Re- Issued Final
1	2013/11/13	Final
0	2013/08/13	Issued for Client Review
BY	DATE	DESCRIPTION

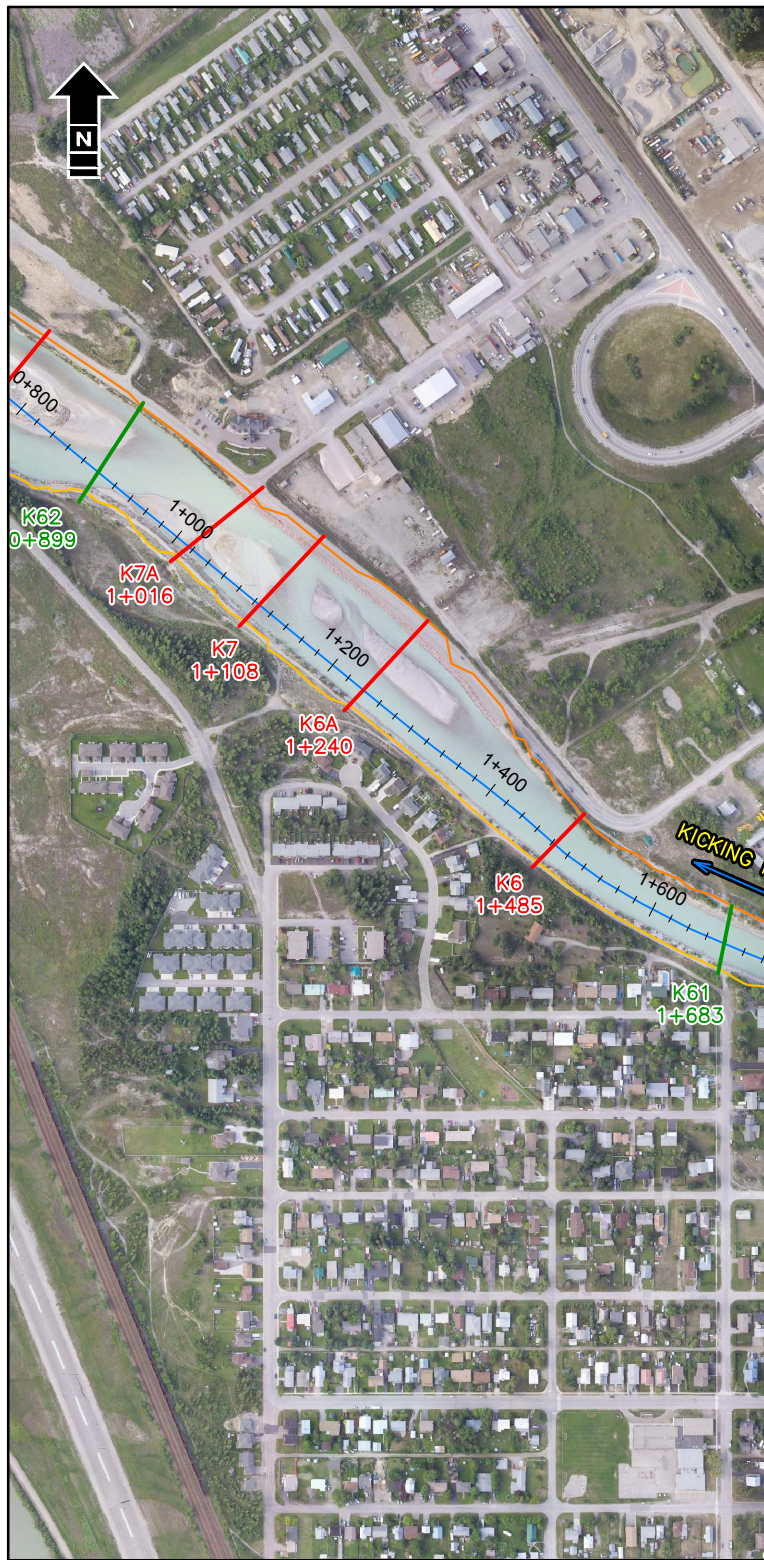
**Town of Golden**  
Flood Risk Assessment Mapping

**Low Freeboard Areas <0.6m**

Date: February 2014	Project: 5635-SP-C3D-14	Technical: J. Bigelow	Reviewer: D. Cooper	Drawn: Z. Steele
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River Station (m)	UTM 11N (m)		Design Grade (m)	Fill Required (m)
	Easting	Northing		
1959	502297	5683147	788.59	0.00
1982			788.66	0.45
1995			788.68	0.80
2005			788.70	0.85
2027			788.74	0.78
2048			788.77	0.60
2070			788.81	0.63
2090			788.97	0.81
2130			789.20	0.83
2148			789.41	0.88
2160			789.51	0.65
2171	502501	5683089	789.62	0.00

River Station (m)	UTM 11N (m)		Design Grade (m)	Fill Required (m)
	Easting	Northing		
1876	502203	5683126	788.36	0.00
1886			788.39	0.16
1896			788.42	0.26
1916			788.48	0.23
1950			788.58	0.30
1962			788.61	0.23
1977			788.66	0.26
1992			788.68	0.20
2009			788.71	0.17
2026			788.73	0.16
2052			788.77	0.24
2066			788.79	0.17
2071			788.80	0.18
2080			788.81	0.02
2092			788.91	0.00
2102			788.99	0.26
2129			789.19	0.43
2155			789.39	0.71
2172			789.53	0.82
2189			789.66	0.14
2211	502505	5682985	789.80	0.00

River Station (m)	UTM 11N (m)		Design Grade (m)	Fill Required (m)
	Easting	Northing		
2272	502566	5682977	789.88	0.00
2295			789.91	0.02
2314			789.93	0.11
2343			789.98	0.11
2351			789.99	0.00
2364			790.01	0.00
2380			790.04	0.05
2392	502679	5682941	790.06	0.00
2671	502956	5682810	791.39	0.00
2687			791.41	0.05
2719			791.46	0.11
2753			791.52	0.00
2772			791.63	0.01
2809			791.83	0.00
2829			791.94	0.02
2855	503161	5682839	792.08	0.00

River Station (m)	UTM 11N (m)		Design Grade (m)	Fill Required (m)
	Easting	Northing		
3316	503590	5682961	794.53	0.00
3329			794.59	0.10
3340			794.64	0.16
3361			794.75	0.17
3375			794.82	0.20
3383			794.86	0.18
3392			794.90	0.18
3397			794.93	0.09
3411			795.00	0.09
3418			795.03	0.09
3434			795.11	0.05
3437			795.13	0.05
3456			795.22	0.16
3476			795.32	0.16
3494	503765	5682974	795.41	0.10

- New Section - Not Used in 2004 Study - Used in Present Study - Surveyed in October 2013
- Section Used in 2004 Study - Re-Surveyed in October 2013
- Surveyed Right Bank - June 2013
- Surveyed Left Bank - June 2013
- | Recommended 1.0 m Freeboard Areas

Note: The areas denoted on this figure indicate areas where 1.0 m freeboard is recommended above the 200 year flood event. The tables provide the fill required for each area to achieve 1.0 m freeboard.



Reference:

STAMP	PERMIT	REVISION	
	4	2014/02/10	Revised Final DLC
	3	2014/01/23	Re- Issued Final DLC
	2	2014/01/06	Re- Issued Final DLC
	1	2013/11/13	Final JB
	0	2013/08/13	Issued for Client Review DLC
No.	DATE	DESCRIPTION	BY



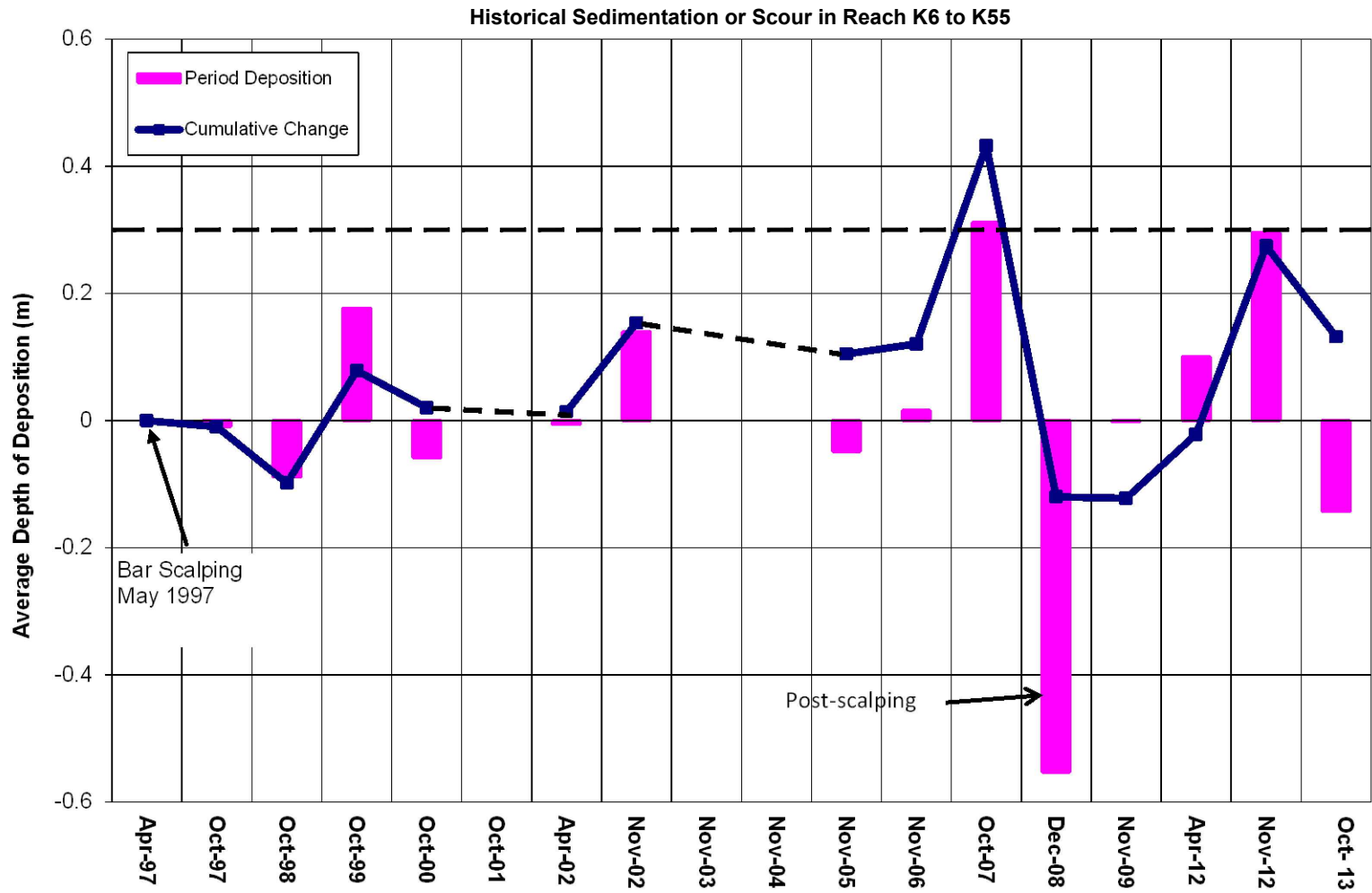
Town of Golden  
Flood Risk Assessment Mapping

### Recommended 1.0 m Priority Freeboard Areas

Date: February 2014 Project: 5835-SP-C3D-14 Technical: J. Bigelow Reviewer: D. Cooper Drawn: C. Zhang

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		2	2014/01/06	Re- Issued Final	DLC
		1	2013/11/13	Final	JB
		0	2013/08/13	Issued for Client Review	DLC
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Town of Golden  
Flood Risk Assessment Mapping

## Historical Sedimentation or Scour in Reach K6 to K55

Date: February 2014    Project: 5635-SP-C3D-14    Technical: J. Bigelow    Reviewer: D. Cooper    Drawn: Z. Steele

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APPENDIX A

## HEC-RAS INPUT AND OUTPUT DATA

KHRNov2013.rep

HEC-RAS Version 4.1.0 Jan 2010  
 U.S. Army Corps of Engineers  
 Hydrologic Engineering Center  
 609 Second Street  
 Davis, California

```

X   X   XXXXXX   XXXX   XXXX   XX   XXXX
X   X   X       X   X   X   X   X   X
X   X   X       X   X   X   X   X   X
XXXXXXXX   XXXX   X   XXX   XXXX   XXXXXX   XXXX
X   X   X       X   X   X   X   X   X
X   X   X       X   X   X   X   X   X
X   X   XXXXXX   XXXX   X   X   X   X   XXXXX
    
```

PROJECT DATA  
 Project Title: KHR Nov2013  
 Project File : KHRNov2013.prj  
 Run Date and Time: 1/22/2014 11:00:39 AM

Project in SI units

Project Description:  
 Kicking Horse River at Golden - Cross sections from October 2013 surveys.

PLAN DATA

Plan Title: Plan 30  
 Plan File : f:\5635\2013 Nov Update\Hecras\KHRNov2013.p30

Geometry Title: Oct 2013  
 Geometry File : f:\5635\2013 Nov Update\Hecras\KHRNov2013.g29

Flow Title : Golden Flows  
 Flow File : f:\5635\2013 Nov Update\Hecras\KHRNov2013.f02

Plan Summary Information:

Number of: Cross Sections = 30 Multiple Openings = 0  
 Culverts = 0 Inline Structures = 0  
 Bridges = 3 Lateral Structures = 0

Computational Information

Water surface calculation tolerance = 0.003  
 Critical depth calculation tolerance = 0.003  
 Maximum number of iterations = 20  
 Maximum difference tolerance = 0.1  
 Flow tolerance factor = 0.001

Computation Options

Critical depth computed only where necessary  
 Conveyance Calculation Method: At breaks in n values only  
 Friction Slope Method: Average Conveyance  
 Computational Flow Regime: Subcritical Flow

KHRNov2013.rep

FLOW DATA

Flow Title: Golden Flows  
 Flow File : f:\5635\2013 Nov Update\Hecras\KHRNov2013.f02

Flow Data (m3/s)

River	Reach	RS	2-Yr-Flood	5-Yr-Flood
10-Yr-Flood	20-Yr-Flood	50-Yr-Flood	100 - Yr-Flood	200-Yr Flood
Columbia	US	2	428	528
586	638	698	741	777
861	Downstream	4	673	834
937	1035	1159	1255	1347
1493	Thalweg_AllSurve	3583	245	306
351	397	461	514	570
632				

Boundary Conditions

River	Reach	Profile	Upstream
Downstream			
Columbia	Downstream	2-Yr-Flood	
Normal S = 0.001	Downstream	5-Yr-Flood	
Columbia	Downstream	10-Yr-Flood	
Normal S = 0.001	Downstream	20-Yr-Flood	
Columbia	Downstream	50-Yr-Flood	
Normal S = 0.001	Downstream		

GEOMETRY DATA

Geometry Title: Oct 2013  
 Geometry File : f:\5635\2013 Nov Update\Hecras\KHRNov2013.g29

Reach Connection Table

River	Reach	Upstream Boundary	Downstream Boundary
Columbia	US		Junction
Columbia	Downstream	Junction	
Thalweg_AllSurve	Thalweg_AllSurve		Junction

JUNCTION INFORMATION

Name: Junction  
Description:  
Energy computation Method

Length across Junction	Tributary	Reach	Length	Angle
River	River	Reach		
Thalweg_AllSurve	Thalweg_AllSurve	to Columbia	0	0
Columbia	US	to Columbia	0	0

CROSS SECTION

RIVER: Columbia  
REACH: US RS: 2

INPUT  
Description: X-Section C6b - South of Golden

Station Elevation Data num= 29									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
3000	783.763000.079	781.9	3001.25	780.77	3001.71	780.9313003.249	780.23		
3005.371	779.233008.361	778.4293009.281	778.3313011.522	778.331	3015.24	778.13			
3020.01	777.883021.601	777.88	3024.46	777.831	3027.13	778.029	3035.01	778.231	
3038.92	778.4293043.458	778.38	3047.4	778.383049.311	778.4813050.569	778.529			
3052.389	778.733053.849	778.9293060.021	779.9193062.981	780.73064.542	781.309				
3065.752	781.9493065.971	782.2113067.879	782.83072.963	782.781					

Manning's n Values num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
3000	.022	3000	.022	3067.879	.022

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
3000	3067.879		109.999	120	124.998	.1		.3

CROSS SECTION

RIVER: Columbia  
REACH: US RS: 1

INPUT  
Description: X-Section C6 - South of Golden

Station Elevation Data num= 29									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
3000	783.763000.079	781.9	3001.25	780.77	3001.71	780.9313003.249	780.23		
3005.371	779.233008.361	778.4293009.281	778.3313011.522	778.331	3015.24	778.13			
3020.01	777.883021.601	777.88	3024.46	777.831	3027.13	778.029	3035.01	778.231	
3038.92	778.4293043.458	778.38	3047.4	778.383049.311	778.4813050.569	778.529			
3052.389	778.733053.849	778.9293060.021	779.9193062.981	780.73064.542	781.309				
3065.752	781.9493065.971	782.2113067.879	782.83073.231	782.781					

Manning's n Values num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
3000	.022	3000	.022	3067.879	.022

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
3000	3067.879		0	0	0	.1		.3

CROSS SECTION

RIVER: Columbia  
REACH: Downstream RS: 4

INPUT  
Description: K55 (2012)

Station Elevation Data num= 63											
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	797.199	4.591	791.504	6.266	790.612	17.89	779.424	32.442	779.32		
40.56	779.902	57.324	779.915	67.941	779.876	75.19	779.682	102.884	780.403		
107.585	780.459	117.242	780.782	118.931	781.055	121.962	780.898	122.528	780.943		
125.444	780.926	128.731	781.171	129.302	781.23	130.581	781.338	130.742	781.316		
131.827	781.657	137.286	781.449	138.65	781.361	139.691	781.274	140.193	781.737		
140.317	781.974	142.626	781.426	143.163	781.005	154.061	780.61	154.748	780.585		
165.261	780.642	167.682	780.985	169.883	780.961	174.989	781.267	179.697	781.584		
182.11	781.616	182.264	781.603	188.488	781.851	191.119	781.929	201.33	781.793		
205.358	781.84	212.164	781.975	229.906	782.371	243.746	782.385	251.287	782.196		
251.387	782.193	261.785	781.65	261.971	781.647	272.607	781.564	288.242	781.12		
288.473	781.127	293.967	781.245	297.782	780.802	297.868	780.805	297.912	780.808		
297.921	780.816	305.839	782.313	319.141	782.712	319.388	782.685	323.283	782.233		
326.589	782.546	329	783.901	329.258	784.046						

Manning's n Values num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
0	.022	0	.022	329	.022

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
0		329		354	354		.1	.3

CROSS SECTION

RIVER: Columbia  
REACH: Downstream RS: 3

INPUT  
Description: X-Section C7 (2003)

Station Elevation Data num= 40											
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
3000	783.1013001.341	781.449	3003.06	780.069	3004.24	779.959	3008.05	778.56			
3011.241	777.563012.439	777.661	3020.15	778.063026.329	778.4593028.819	778.261					
3031.041	778.161	3037.92	778.761	3040.1	778.959	3043.87	778.9593045.851	778.859			
3052.691	778.4593053.959	778.3593056.339	778.663057.979	778.66	3061.04	778.56					
3063.722	778.761	3066.45	778.9593073.481	780.3893076.691	780.971	3080.58	781.111				
3082.369	781.48	3082.89	781.65	3086.99	781.7513089.111	781.2913091.321	781.169				
3094.202	781.263096.411	781.93098.691	781.8793104.979	783.2293109.938	782.059						
3115.669	782.291	3117.72	783.473126.501	783.5743126.949	783.583129.031	782.729					

Manning's n Values num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
3000	.022	3000	.022	3126.501	.022

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
3000	3126.501		156	156	156		.1	.3

CROSS SECTION

RIVER: Columbia  
REACH: Downstream RS: 2

INPUT  
Description:

Station Elevation Data num= 49											
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
3000	787.173022.001	785.4213023.991	785.143024.241	783.793024.841	783.79						

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Table with 8 columns: Station, n, Val, Sta, n, Val, Sta, n, Val. Contains data for stations 3024.841 through 3128.47.

Manning's n Values table with columns: Sta, n, Val, Sta, n, Val. Values: 3000, .022, 3023.991, .022, 3128.47, .022.

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan. Values: 3023.991, 3128.47, 6, 6, Right, .1, .3.

BRIDGE

RIVER: Columbia REACH: Downstream RS: 1.5

INPU Description: Columbia River Bridge - SW of Golden Distance from Upstream XS = 1 Deck/Roadway width = 4.999 Weir Coefficient = 1.44 Upstream Deck/Roadway Coordinates

Upstream Deck/Roadway Coordinates table with columns: Sta, Hi, Cord, Lo, Cord, Sta, Hi, Cord, Lo, Cord. Values: 3020, 785.4, 784.2, 3130, 785.4, 784.2.

Upstream Bridge Cross Section Data

Station Elevation Data table with 8 columns: Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev. Contains data for stations 3024.841 through 3128.47.

Manning's n Values table with columns: Sta, n, Val, Sta, n, Val, Sta, n, Val. Values: 3000, .022, 3023.991, .022, 3128.47, .022.

Bank Sta: Left Right Coeff Contr. Expan. Values: 3023.991, 3128.47, .1, .3.

Downstream Deck/Roadway Coordinates table with columns: Sta, Hi, Cord, Lo, Cord, Sta, Hi, Cord, Lo, Cord. Values: 3020, 785.4, 784.2, 3130, 785.4, 784.2.

Downstream Bridge Cross Section Data

Station Elevation Data table with 8 columns: Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev. Values: 3000, 786.881, 3023.54, 785.381, 3025.701, 785.15, 3026, 783.781, 3026.5, 783.812.

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Table with 8 columns: Station, n, Val, Sta, n, Val, Sta, n, Val. Contains data for stations 3026.5 through 3170.2.

Manning's n Values table with columns: Sta, n, Val, Sta, n, Val. Values: 3000, .022, 3025.701, .022, 3130.198, .022.

Bank Sta: Left Right Coeff Contr. Expan. Values: 3025.701, 3130.198, .1, .3.

Upstream Embankment side slope = 3.73 horiz. to 1.0 vertical Downstream Embankment side slope = 3.73 horiz. to 1.0 vertical Maximum allowable submergence for weir flow = .95 Elevation at which weir flow begins = Energy head used in spillway design = Spillway height used in design = Weir crest shape = Broad Crested

Number of Piers = 4

Pier Data table for Pier Station 3040. Columns: Pier Station, Upstream, Downstream, num, width, Elev. Values: 3040, 3040, 2, 1, 780.919, 1, 789.21.

Pier Data table for Pier Station 3065. Columns: Pier Station, Upstream, Downstream, num, width, Elev. Values: 3065, 3065, 2, 1, 780.919, 1, 789.21.

Pier Data table for Pier Station 3090. Columns: Pier Station, Upstream, Downstream, num, width, Elev. Values: 3090, 3090, 2, 1, 780.919, 1, 789.21.

Pier Data table for Pier Station 3115. Columns: Pier Station, Upstream, Downstream, num, width, Elev. Values: 3115, 3115, 2, 1, 780.919, 1, 789.21.

1 780.919 1 789.21

Number of Bridge Coefficient Sets = 1

Low Flow Methods and Data

Energy Selected Low Flow Methods = Highest Energy Answer

High Flow Method Energy Only

Additional Bridge Parameters

- Add Friction component to Momentum
Do not add weight component to Momentum
Class B flow critical depth computations use critical depth inside the bridge at the upstream end
Criteria to check for pressure flow = Upstream energy grade line

CROSS SECTION

RIVER: Columbia REACH: Downstream RS: 1

INPUT

Description: X-Section C57 : Downstream of Bridge (2003)

Table with 12 columns: Station, Elev, Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev. Contains elevation data for station 3000 to 3127.

Manning's n Values table with columns: Sta, n Val, Sta, n Val, Sta, n Val. Values: 3000 .0223025.701 .0223130.198 .022

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan. Values: 3025.7013130.198 130.159 132.161 135.161 .1 .3

CROSS SECTION

RIVER: Columbia REACH: Downstream RS: 0

INPUT

Description: X-Section C57.0 Downstream X Section C57

Table with 12 columns: Station, Elev, Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev. Contains elevation data for station 3000 to 3059.

Table with 12 columns: Station, Elev, Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev. Contains elevation data for station 3076.2 to 3183.341.

Manning's n Values table with columns: Sta, n Val, Sta, n Val, Sta, n Val. Values: 3000.601 .022 3030.84 .022 3135.34 .022

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan. Values: 3030.84 3135.34 0 0 0 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve REACH: Thalweg\_AllSurve RS: 3583

INPUT

Description: X-Section K1 - Oct 2013

Table with 12 columns: Station, Elev, Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev. Contains elevation data for station 2.22 to 39.63.

Manning's n Values table with columns: Sta, n Val, Sta, n Val, Sta, n Val. Values: 0 .032 0 .032 39.17 .032

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan. Values: 0 39.17 72 65 59 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve REACH: Thalweg\_AllSurve RS: 3519

INPUT

Description: X-Section K52 - Oct 2013

Table with 12 columns: Station, Elev, Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev. Contains elevation data for station 9.21 to 56.74.

Manning's n Values table with columns: Sta, n Val, Sta, n Val, Sta, n Val. Values: 0 .032 0 .032 60.07 .032



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Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
0 60.07 381 386 391 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 3132

INPUT  
Description: Section K2 - Oct 2013  
Station Elevation Data num= 42  
Sta Elev Sta Elev Sta Elev Sta Elev  
0 793.8 4.88 793.69 5.19 793.597 11.07 791.845 11.6 791.725  
14.33 791.118 18.94 791.05 21.45 791.012 22.86 790.75 24.65 790.422  
27.2 790.269 27.98 790.222 29.52 790.375 29.56 790.379 29.59 790.374  
30.78 790.162 30.96 790.16 34.35 790.115 40.18 789.959 42.73 789.891  
43.6 789.977 44.9 790.105 45.59 789.998 46.32 789.887 48.63 789.832  
50.88 789.778 53.37 789.782 54.87 789.784 56.16 789.798 59.16 789.829  
60.58 789.729 61.52 789.664 62.5 789.698 63.58 789.735 65.02 789.636  
66.42 789.537 67.46 789.541 68.14 789.544 68.5 789.908 69.12 790.5  
72.51 792.571 76.91 795.211

Manning's n Values num= 3  
Sta n Val Sta n Val Sta n Val  
0 .032 4.88 76.91 .032

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
4.88 76.91 385 390 377 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 2742

INPUT  
Description: K50 - Oct 2013  
K50  
Station Elevation Data num= 73  
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
0 791.6 3.15 791.092 3.16 791.092 3.18 791.093 4.39 790.698  
6.65 789.944 8.09 789.063 8.51 788.82 8.65 788.714 9.04 788.362  
9.84 788.341 10.5 788.321 11.52 788.428 12.09 788.486 15.22 788.59  
22.45 788.831 24.47 788.944 28.69 789.181 29.93 789.216 31.71 789.266  
33.37 789.204 34.87 789.149 36.82 788.922 38.41 788.74 39.48 788.758  
40.57 788.776 41.46 789.673 41.91 790.113 50.25 790.099 60.91 790.081  
61.36 789.495 62.15 788.528 62.91 788.28 63.27 788.164 64.93 788.1  
68.44 787.963 69.44 787.767 70.14 787.63 72.13 787.68 73.63 787.718  
74.57 787.75 76.53 787.816 78.14 788.064 78.79 788.159 79.89 788.385  
80.72 788.558 80.97 788.607 81.99 788.837 83.24 788.932 93.65 789.724  
96.07 789.662 100.68 789.545 104.06 789.215 104.29 789.192 104.95 789.17  
110.58 788.98 115.46 788.628 116.52 788.552 116.9 788.545 119.48 788.5  
120.3 788.588 124.01 788.982 124.51 789.335 125.47 789.91 126.23 790.933  
126.29 791.018 126.38 791.01 127.69 790.877 127.71 790.877 127.96 790.873  
128.22 790.869 139.31 791.079 140 792.3

Manning's n Values num= 3  
Sta n Val Sta n Val Sta n Val  
0 .032 4.44 65.55 .032

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
4.44 65.55 133 128 115 .1 .3

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0 140 217 171 157 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 2571

INPUT  
Description: K60 - Oct 2013  
Station Elevation Data num= 75  
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
4.47 790.928 4.69 790.944 4.88 790.946 5.08 790.948 5.16 790.949  
6.57 790.188 8.68 789.105 9.86 788.548 11.61 787.737 12.08 787.615  
12.81 787.389 19.18 787.678 20.93 787.757 21.22 787.766 23.96 787.855  
24.87 787.885 24.96 787.888 25.2 787.883 32.33 787.718 35.16 787.742  
35.53 787.745 35.73 787.723 40.73 787.206 42.44 787.06 42.74 787.034  
43.58 787.176 45.77 787.552 46.55 787.79 46.65 787.827 47.67 788.284  
48.38 788.603 62.59 788.772 63.44 788.782 63.64 788.781 63.82 788.779  
65.47 788.767 66.42 788.761 66.62 788.759 77.19 788.71 78.44 788.704  
80.39 788.221 82.12 787.791 84.13 787.551 84.27 787.539 84.29 787.535  
84.74 787.482 86.67 787.412 87.43 787.389 94.79 786.759 96.85 786.581  
97.43 786.59 98.88 786.612 101.2 786.545 103.49 786.48 105.89 786.623  
108.38 786.765 109.76 787.049 109.92 787.081 109.99 787.106 110.85 787.409  
111.46 787.565 111.74 787.679 112.11 787.748 113.06 787.887 113.19 787.907  
113.26 787.916 115 788.851 115.54 789.195 116.6 789.696 119.06 792.002  
119.17 790.729 119.49 790.718 119.81 790.707 119.82 790.708 129.04 790.354

Manning's n Values num= 3  
Sta n Val Sta n Val Sta n Val  
4.47 .032 4.47 .032 119.06 .032

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
4.47 119.06 133 128 115 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 2443

INPUT  
Description: K51 - Oct 2013  
Station Elevation Data num= 41  
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
0 790.427 4.44 790.463 4.68 790.459 4.81 790.457 8.72 788.177  
9.86 787.499 10.39 787.307 12.66 786.496 13.29 786.144 14.25 785.573  
15.55 785.4 16.02 785.34 17.88 785.367 19.99 785.398 20.39 785.434  
21.4 785.526 25.67 785.793 25.74 785.798 25.79 785.799 32.62 785.89  
35.77 786.111 36.09 786.133 36.27 786.145 38.16 786.274 44.08 786.428  
44.24 786.432 44.34 786.434 46.69 786.485 48.9 786.533 49 786.535  
49.02 786.537 49.96 786.614 50.31 786.781 52.24 787.646 53.68 788.522  
55.08 789.359 55.13 789.362 55.32 789.374 55.51 789.386 55.92 789.373  
65.55 789.849

Manning's n Values num= 3  
Sta n Val Sta n Val Sta n Val  
0 .032 4.44 .032 65.55 .032

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
4.44 65.55 133 128 115 .1 .3

CROSS SECTION

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RIVER: Thalweg\_AllSurve  
 REACH: Thalweg\_AllSurve RS: 2312

INPUT

Description: K4 - Oct 2013

Station Elevation Data		num= 42		Sta		Elev		Sta		Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	789.806	6.36	789.62	6.79	789.611	7.06	789.606	11.64	787.712		
11.71	787.686	11.81	787.667	19.34	786.314	23.18	786.272	24.23	786.26		
27	786.118	27.3	786.103	28.79	786.036	36.09	785.709	38.61	785.562		
40.11	785.475	40.98	785.415	42.88	785.284	45.46	785.28	46.14	785.279		
48.28	785.398	48.51	785.411	49.14	785.419	53.5	785.473	55.68	785.554		
56.09	785.569	56.69	785.559	59.69	785.51	62.63	785.535	62.93	785.538		
63.03	785.539	64.34	785.549	64.64	785.552	64.66	785.552	65.61	786.004		
65.64	786.018	66.13	786.317	71.18	789.444	71.28	789.445	71.61	789.449		
71.9	789.455	73	789.8								

Manning's n Values		num= 3		Sta		n Val	
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.032	0	.032	73	.032		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
0		73		125	131		.1	.3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
 REACH: Thalweg\_AllSurve RS: 2213

INPUT

Description: K10 Oct 2013

Station Elevation Data		num= 67		Sta		Elev		Sta		Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	789.816	5	790.174	5.87	790.236	5.97	790.286	6.6	790.276		
7.16	790.225	7.3	790.264	7.31	790.266	7.95	790.24				
13.84	787.056	14.45	786.714	15.06	786.535	15.32	786.46	19.12	786.354		
19.73	786.337	23.83	786.325	24.28	786.324	24.74	786.496	27.9	787.657		
29.63	787.85	30.18	787.912	32.04	788.224	35.37	788.787	38.56	788.902		
41.36	789.004	44.53	788.525	49.13	787.833	55.96	787.444	56.13	787.434		
56.19	787.423	59.2	786.851	60.7	786.286	62.03	785.813	62.77	785.753		
63.36	785.703	64.69	785.392	65.34	785.238	66.41	785.139	69.01	784.901		
70.78	784.764	71.48	784.714	74.2	784.695	75.07	784.689	79.11	784.527		
80.39	784.477	81.15	784.434	83.19	784.319	84.34	784.248	86.73	784.249		
90.66	784.25	92.15	784.343	93.18	784.407	95.45	784.528	97.12	784.612		
98.9	785.151	98.94	785.162	99.03	785.206	102.54	786.868	104.96	787.8		
107.33	788.736	110.29	789.726	112.08	790.323	112.14	790.243	112.38	789.918		
112.59	789.637	113.61	790.256								

Manning's n Values		num= 3		Sta		n Val	
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.032	5.87	.032	112.08	.032		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
5.87		112.08		16	29		.1	.3

BRIDGE

RIVER: Thalweg\_AllSurve  
 REACH: Thalweg\_AllSurve RS: 2212.8

INPUT

Description:

Distance from Upstream XS = 1.329

Deck/Roadway Width = 8.199

Weir Coefficient = 1.44

Upstream Deck/Roadway Coordinates

num= 8		Sta		Hi Cord		Lo Cord		Sta		Hi Cord		Lo Cord	
Sta	Hi Cord	Lo Cord	Sta	Hi Cord	Lo Cord	Sta	Hi Cord	Lo Cord	Sta	Hi Cord	Lo Cord	Sta	Hi Cord
8	790.581	786	14	790.581	789.11	28.63	790.581	789.11	61.8	790.569	786	61.8	790.569
29.73	790.581	786	61.8	790.569	786	109	790.569	786.671					
105.089	790.569	788.82											

Upstream Bridge Cross Section Data

Station Elevation Data		num= 67		Sta		Elev		Sta		Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	789.816	5	790.174	5.87	790.236	5.97	790.286	6.6	790.276		
7.16	790.225	7.3	790.264	7.31	790.266	7.95	790.24				
13.84	787.056	14.45	786.714	15.06	786.535	15.32	786.46	19.12	786.354		
19.73	786.337	23.83	786.325	24.28	786.324	24.74	786.496	27.9	787.657		
29.63	787.85	30.18	787.912	32.04	788.224	35.37	788.787	38.56	788.902		
41.36	789.004	44.53	788.525	49.13	787.833	55.96	787.444	56.13	787.434		
56.19	787.423	59.2	786.851	60.7	786.286	62.03	785.813	62.77	785.753		
63.36	785.703	64.69	785.392	65.34	785.238	66.41	785.139	69.01	784.901		
70.78	784.764	71.48	784.714	74.2	784.695	75.07	784.689	79.11	784.527		
80.39	784.477	81.15	784.434	83.19	784.319	84.34	784.248	86.73	784.249		
90.66	784.25	92.15	784.343	93.18	784.407	95.45	784.528	97.12	784.612		
98.9	785.151	98.94	785.162	99.03	785.206	102.54	786.868	104.96	787.8		
107.33	788.736	110.29	789.726	112.08	790.323	112.14	790.243	112.38	789.918		
112.59	789.637	113.61	790.256								

Manning's n Values		num= 3		Sta		n Val	
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.032	5.87	.032	112.08	.032		

Bank Sta:	Left	Right	Coeff	Contr.	Expan.
5.87		112.08		.1	.3

Downstream Deck/Roadway Coordinates

num= 8		Sta		Hi Cord		Lo Cord	
Sta	Hi Cord	Lo Cord	Sta	Hi Cord	Lo Cord	Sta	Hi Cord
0	790.581	786	8	790.581	789.11	22.6	790.581
23.66	790.581	786	66.95	790.569	786	66.96	790.569
109	790.569	788.82	113	790.569	786.671		

Downstream Bridge Cross Section Data

Station Elevation Data		num= 62		Sta		Elev		Sta		Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	789.5	.5	787.344	3.74	788.518	3.92	788.575	4.14	788.486		
10.86	785.773	18.96	785.982	19.09	785.985	19.14	786.024	22.57	788.312		
22.61	788.319	23.14	788.386	23.66	788.451	23.72	788.458	25.99	788.251		
29.63	787.921	34.7	787.443	34.89	787.426	35.1	787.406	40.71	787.395		
62.11	787.353	65.89	787.723	66.95	787.827	67.19	787.692	68.9	786.724		
69.7	786.547	72.61	785.98	72.62	785.977	72.63	785.976	72.63	785.975		
72.87	785.577	74.23	785.081	74.44	784.992	75.07	784.922	77.24	784.681		
78.54	784.6	79.33	784.551	83.66	784.23	83.73	784.225	83.81	784.225		
87.43	784.221	89.04	784.193	90.77	784.164	91.82	784.252	92.04	784.272		
93.39	784.184	94.95	784.084	95.17	784.072	96.15	784.022	96.89	784.028		
97.64	784.035	99.01	784.199	99.33	784.236	99.72	784.246	103.2	784.336		
103.72	784.349	106.42	784.417	107.13	784.72	107.81	785.033	109.1	785.779		
111.75	787.369	113	789.5								

Manning's n Values		num= 3		Sta		n Val	
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val

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Sta	n Val	Sta	n Val	Sta	n Val
0	.032	0	.032	113	.032

Bank Sta: Left Right Coeff Contr. Expan.  
0 113 .1 .3

Ineffective Flow num= 1  
Sta L Sta R Elev Permanent  
23.66 66.95 790.5 T

Upstream Embankment side slope = 1.72 horiz. to 1.0 vertical  
Downstream Embankment side slope = 0 horiz. to 1.0 vertical  
Maximum allowable submergence for weir flow = .95  
Elevation at which weir flow begins =  
Energy head used in spillway design =  
Spillway height used in design =  
Weir crest shape = Broad Crested

Number of Bridge Coefficient Sets = 1

Low Flow Methods and Data  
Energy  
Selected Low Flow Methods = Highest Energy Answer

High Flow Method  
Energy Only

Additional Bridge Parameters  
Add Friction component to Momentum  
Do not add Weight component to Momentum  
Class B flow critical depth computations use critical depth  
inside the bridge at the upstream end  
Criteria to check for pressure flow = Upstream energy grade line

CROSS SECTION

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 2184

INPUT  
Description: Section K11 Oct 2013  
Station Elevation Data num= 62

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	789.5	.5	787.344	3.74	788.518	3.92	788.575	4.14	788.486
10.86	785.773	18.96	785.982	19.09	785.985	19.14	786.024	22.57	788.312
22.61	788.319	23.14	788.386	23.66	788.451	23.72	788.458	25.99	788.251
29.63	787.921	34.7	787.443	34.89	787.426	35.1	787.406	40.71	787.395
62.11	787.353	65.89	787.723	66.95	787.827	67.19	787.692	68.9	786.724
69.7	786.547	72.61	785.98	72.62	785.977	72.63	785.976	72.63	785.975
72.87	785.577	74.23	785.081	74.44	784.992	75.07	784.922	77.24	784.681
78.54	784.6	79.33	784.551	83.66	784.23	83.73	784.225	83.81	784.225
87.43	784.221	89.04	784.193	90.77	784.164	91.82	784.252	92.04	784.272
93.39	784.184	94.95	784.084	95.17	784.072	96.15	784.022	96.89	784.028
97.64	784.035	99.01	784.199	99.33	784.236	99.72	784.246	103.2	784.336
103.72	784.349	106.42	784.417	107.13	784.72	107.81	785.033	109.1	785.779
111.75	787.369	113	789.5						

Manning's n Values num= 3  
Sta n Val Sta n Val  
0 .032 0 .032 113 .032

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
0 113 147 117 112 .1 .3

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Ineffective Flow	num=	1
Sta L Sta R Elev Permanent		T
23.66 66.95 790.5		

CROSS SECTION

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 2068

INPUT  
Description: Section K11b Oct 2013

Station Elevation Data num= 72

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	789.362	7.33	789.86	8.69	790.498	8.99	790.536	9.28	790.536
9.45	790.536	9.76	790.536	9.84	790.328	10.28	788.561	10.44	788.297
11.4	788.206	11.65	788.162	16.94	786.208	16.96	786.2	17.28	786.237
18.45	786.375	18.5	786.374	23.98	786.278	26.19	787.004	27.43	787.405
27.49	787.726	28	789.075	28.57	790.367	28.65	790.551	29.94	790.497
30.74	790.463	39.99	790.417	41.32	790.41	42.66	790.418	51.8	790.472
52.97	790.485	58.37	790.545	58.63	789.207	59.78	788.745	59.89	788.55
59.91	788.551	59.97	788.528	62.07	787.725	62.08	787.723	63.64	787.514
69.83	785.698	69.98	785.655	70.01	785.654	71.07	785.623	71.24	785.57
72.63	785.138	73.4	784.913	73.51	784.879	74.15	784.834	78.55	784.522
84.85	784.273	85.91	784.232	86.23	784.211	88.01	784.094	89.43	784.184
89.77	784.205	90.28	784.19	92.18	784.132	94.93	784.371	95.31	784.404
96.37	784.492	97.23	784.564	97.42	784.564	99.08	784.565	100.14	784.795
101.85	785.14	101.92	785.137	102.04	785.133	104.16	785.547	105.78	785.836
106.1	785.845	111.35	789.943						

Manning's n Values num= 3  
Sta n Val Sta n Val Sta n Val  
0 .032 8.69 .032 111.35 .032

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
8.69 111.35 92 97 96 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 1971

INPUT  
Description: K5 - Oct 2013  
Station Elevation Data num= 65

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	788.5	7.37	787.826	7.84	787.838	8.31	787.85	8.43	787.853
11.68	786.521	12.11	786.349	12.49	786.262	15.09	785.694	15.64	785.465
15.74	785.417	16.13	785.389	17.65	785.273	18.76	785.189	19.01	785.169
19.14	785.169	20.38	785.17	20.91	785.394	21.49	785.634	22.12	785.844
24.72	786.626	25.05	787.093	25.15	787.257	25.58	787.357	27.17	787.805
27.44	787.877	27.49	787.894	29	787.895	29.02	787.895	29.04	787.882
29.38	787.677	30.45	787.027	30.52	786.987	30.67	786.94	32.71	786.297
34.29	785.606	35.06	785.256	35.82	785.025	37.61	784.517	38.53	784.3
39.99	783.944	41.9	783.984	41.97	783.986	43.05	783.994	46.08	784.017
46.36	784.002	50.67	783.779	53.99	783.792	54.91	783.796	56.21	783.839
60.72	783.989	62.19	784.036	68.2	784.226	68.35	784.409	68.46	784.566
68.5	784.591	69.46	785.159	71.45	785.55	72.53	785.781	76.72	788.041
77.01	788.199	77.02	788.199	77.18	788.198	77.34	788.197	77.8	788.415

Manning's n Values num= 3  
Sta n Val Sta n Val  
0 .032 111.35 .032 111.35 .032

0 .025 0 .025 KHRNov2013.rep  
77.8 .025

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
0 77.8 296 292 287 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 1679

INPUT  
Description: K61 - Oct 2013  
Station Elevation Data num= 47

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	787.014	3.701	787.23	4.74	787.29	4.86	787.294
5.02	787.299	5.7	787.083	6.43	786.865	10.63	784.783
12.08	784.045	12.31	783.737	14.19	783.469	15.34	783.284
19.46	783.429	24.73	783.467	25.41	783.472	27.31	783.457
33.78	783.466	37.48	783.492	43.23	783.518	43.63	783.52
47.64	783.64	48.05	783.884	48.13	783.938	48.29	783.962
49.3	784.178	49.61	784.423	50.52	784.603	51.41	784.777
55.8	785.195	55.94	785.229	57.9	785.649	58.03	785.687
60.84	786.437	61.4	786.581	62.03	786.893	64.15	787.944
64.66	787.938	67.5	787.923			64.4	787.941

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.025	4.98	.025	64.15	.025

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
4.98 64.15 209 196 183 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 1483

INPUT  
Description: K6 - Oct 2013  
Station Elevation Data num= 24

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	787.8	.53	786.512	4.22	784.936	4.78	784.698
6.24	784.051	6.87	783.704	7.47	783.387	9.77	783.282
19.82	783.183	21.84	783.173	29.98	783.253	40.32	783.354
48.11	783.526	52.02	783.946	52.89	784.04	54.43	784.591
55.33	784.889	57.19	785.465	60.6	787.626	60.85	787.781

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.025	0	.025	60.85	.025

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
0 60.85 244 244 245 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 1239

INPUT

Description: K6A - Oct 2013 KHRNov2013.rep  
Station Elevation Data num= 47

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	786.327	.11	786.325	.12	786.325	3.69	785.173
4.23	784.931	5.58	783.908	6.18	783.649	7.84	782.948
14.93	783.071	16.4	783.117	16.67	783.125	17.16	783.129
20.9	783.164	21	783.165	21.19	783.172	27.25	783.403
31.44	783.704	31.56	783.705	33.57	783.729	37.2	783.938
37.41	783.944	47.13	784.125	57.22	784.113	61.79	784.107
66.51	783.74	68.15	783.903	69.07	783.995	70.85	783.932
78.88	782.868	79.05	782.826	79.1	782.818	83.44	782.145
89.42	782.211	90.02	782.37	91.71	782.82	94.36	785.044
94.8	785.284	98.95	786.771			94.53	785.186

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.025	0	.025	98.95	.025

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
0 98.95 133 133 126 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 1106

INPUT  
Description: K7 - Oct 2013  
Station Elevation Data num= 40

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	786.751	.84	786.297	6.02	783.552	6.63	783.421
7.1	783.344	8.34	783.446	11.22	783.508	15.94	783.608
22.07	783.711	31.18	783.534	34.11	783.477	35.7	783.457
49.7	783.221	52.6	783.156	59.07	782.939	62.41	782.826
67.75	782.706	72.83	782.437	74.91	782.327	77.35	782.116
81.28	781.618	81.42	781.587	81.53	781.569	82.34	781.43
90.62	781.089	91.77	782.068	92.2	782.423	92.56	782.506
95.34	784.239	95.58	784.476	96.2	784.719	101.02	786.627

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.025	0	.025	101.12	.025

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
0 101.12 92 91 87 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 1015

INPUT  
Description: K7A - Oct 2013  
Station Elevation Data num= 73

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	786.025	.11	786.028	4.06	784.374	4.28	784.283
6.81	783.279	7.38	783.246	7.4	783.245	7.44	783.251
8.71	783.407	10.53	783.395	11.4	783.313	11.68	783.286
18.68	783.246	20.77	783.158	22.68	783.077	23.66	783.023
26.34	782.979	26.95	782.992	27.16	782.964	27.68	782.897
29.73	782.838	30.88	782.905	30.91	782.907	30.92	782.905

KHRNov2013.rep											
32.16	782.995	32.26	783.024	32.27	783.027	32.97	783.353	37.06	783.558		
37.3	783.571	37.45	783.569	40.02	783.535	41.43	783.652	42.44	783.735		
47.51	783.681	47.56	783.68	47.6	783.674	48.97	783.502	52.29	783.084		
52.41	783.07	52.44	783.063	53.51	782.78	54.89	782.475	57.32	781.935		
59.15	781.909	63.08	781.854	67	781.658	67.68	781.624	69.67	781.457		
69.91	781.438	69.96	781.434	70.34	781.4	79.89	780.938	81.53	780.855		
81.77	780.914	83.04	781.229	83.33	781.596	83.78	782.357	84.44	782.647		
84.72	782.781	85.28	783.273	85.49	783.445	87.52	784.337	90.94	785.85		
91.07	785.854	91.37	785.863	91.66	785.872						

Manning's n Values num= 3  
 Sta n Val Sta n Val  
 0 .025 0 .025 91.66 .025

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 0 91.66 112 116 143 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
 REACH: Thalweg\_AllSurve RS: 899.86

INPUT  
 Description: K62 - Oct 2013  
 Station Elevation Data num= 58

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
.91	784.77	1.17	784.774	1.47	784.763	1.78	784.753	1.81	784.752
4.34	784.879	6.23	784.989	7.14	785.038	8.2	785.104	9.16	785.133
10.88	785.185	11.68	785.209	12.13	785.222	12.83	784.842	14.38	784.03
15	782.847	15.38	782.172	15.75	782.068	16.1	781.96	17.01	781.595
18.81	780.79	19.19	780.835	20.28	780.973	23.86	781.161	24.12	781.175
25.77	781.26	25.95	781.27	28.46	781.26	29.08	781.257	31.7	781.049
31.7	781.048	31.71	781.051	34.29	781.963	36.43	782.105	36.84	782.132
53.72	782.656	55.71	782.718	57.58	782.734	65.07	782.796	70.75	782.835
81.41	782.909	84.3	782.87	90.41	782.787	96.17	782.704	101.86	782.622
102.77	782.544	103.69	782.467	104.66	782.659	105.39	782.806	105.82	783.116
106.8	783.832	107.27	784.159	109.75	785.594	109.85	785.653	109.87	785.666
111.12	785.706	111.83	785.738	112.39	785.759				

Manning's n Values num= 3  
 Sta n Val Sta n Val  
 .91 .025 12.13 .025 112.39 .025

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 12.13 112.39 138 127 119 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
 REACH: Thalweg\_AllSurve RS: 772

INPUT  
 Description: K7B - Oct 2013  
 Station Elevation Data num= 45

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	784.967	.77	784.842	.78	784.839	4.96	781.819	5.64	780.859
5.69	780.787	7.18	780.573	7.2	780.571	7.23	780.574	9.65	780.795
14.93	781.024	15	781.027	15.04	781.027	18.14	781.002	21.41	781.045
21.45	781.046	26.27	781.773	26.59	781.82	26.82	781.892	29.88	782.854
36.25	783.256	36.41	783.266	36.68	783.264	48.51	783.16	51.63	782.98
51.7	782.976	52	782.984	66.49	783.375	77.05	783.306	77.26	783.305

KHRNov2013.rep											
77.53	783.305	92.24	783.303	92.45	783.293	104.81	782.688	104.99	782.682		
116.13	782.312	116.26	782.308	125	782.007	125.45	782.072	125.83	782.127		
126.02	782.264	126.54	782.629	127.46	782.926	127.49	782.937	131.29	785.313		

Manning's n Values num= 3  
 Sta n Val Sta n Val  
 0 .025 0 .025 131.29 .025

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 0 131.29 133 129 124 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
 REACH: Thalweg\_AllSurve RS: 643.15

INPUT  
 Description: K8 - Oct 2013  
 Station Elevation Data num= 66

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	785.094	.25	784.858	.35	784.832	4.07	783.82	6.51	782.534
6.59	782.491	6.67	782.484	7.43	782.413	9.68	782.204	9.9	782.184
10.79	781.745	10.85	781.714	12.52	781.386	12.54	781.381	12.56	781.373
13.32	780.849	13.87	780.934	14.46	781.026	17.49	781.042	17.61	781.043
20.89	780.749	21	780.739	23.83	780.759	29.57	780.829	29.58	780.829
29.59	780.829	35.59	780.513	35.64	780.515	37.12	780.563	40.53	781.267
40.62	781.287	40.71	781.299	44.1	781.717	44.3	781.744	45.39	781.89
47.11	782.004	47.4	782.024	48.35	781.991	53.84	781.801	55.21	782.063
55.5	782.12	56.09	782.174	58.78	782.418	60.33	782.42	60.64	782.421
60.94	782.442	62.44	782.546	68.24	782.756	68.6	782.769	72.89	782.801
80.43	782.857	90.48	782.766	91.13	782.76	91.46	782.765	96.6	782.85
101.21	782.674	101.52	782.662	101.72	782.647	104.32	782.456	105.39	782.469
105.6	782.471	105.69	782.504	106.17	782.664	110.72	784.943	110.73	784.944
110.87	785.253								

Manning's n Values num= 3  
 Sta n Val Sta n Val  
 0 .025 0 .025 110.87 .025

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 0 110.87 148 179 194 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
 REACH: Thalweg\_AllSurve RS: 463.66

INPUT  
 Description: K53  
 Station Elevation Data num= 32

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	786.869	.3	786.883	.31	786.885	5	786.779	2.94	785.42
7.87	784.767	9.25	784.583	13.87	782.576	14.12	782.466	14.29	782.44
16.9	782.034	21.03	782.201	21.46	782.219	24.74	781.989	24.78	781.986
33.67	781.651	34	781.639	59.94	781.204	61.65	781.175	62.28	781.135
71.39	780.552	75.9	780.621	76.29	780.627	76.52	780.648	78.99	780.876
80.93	781.495	81.33	781.625	83.27	782.216	83.68	782.344	84.44	782.704
87.92	784.34	89.91	784.58						

Manning's n Values num= 3  
 Sta n Val Sta n Val  
 0 .025 0 .025 110.87 .025

0 .025 0 .025  
Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
0 118.4 18 18 18 .1 .3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 410.07

INPUT  
Description: k54 - Oct 2013  
Station Elevation Data num= 36

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	786.874	.06	786.874	.14	786.873	1.71	786.277	5.45	784.859
10.05	784.549	10.09	784.546	10.19	784.522	21.37	781.889	24.3	782.157
24.34	782.16	24.39	782.16	28.43	782.186	60.31	781.655	61.01	781.643
81.39	780.719	81.95	780.693	87.12	780.377	87.27	780.368	87.66	780.366
101.29	780.292	101.31	780.3	102.15	780.561	102.52	780.695	103.84	781.176
107.05	781.537	108.07	781.651	111.39	782.95	111.51	782.998	114.83	783.535
114.95	783.555	115.07	783.599	118.4	784.794	118.85	784.78	119.89	784.749

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.025	0	.025	118.4	.025

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
0 118.4 18 18 18 .1 .3

BRIDGE

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 409

INPUT  
Description:  
Distance from Upstream XS = 1.999  
Deck/Roadway width = 13.5  
Weir Coefficient = 1.4  
Upstream Deck/Roadway Coordinates num= 4

Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord
0	788.45	784.68	0	788.45	785.55	150	788.45	785.55	
150	788.45	784.54							

Upstream Bridge Cross Section Data  
Station Elevation Data num= 36

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	786.874	.06	786.874	.14	786.873	1.71	786.277	5.45	784.859
10.05	784.549	10.09	784.546	10.19	784.522	21.37	781.889	24.3	782.157
24.34	782.16	24.39	782.16	28.43	782.186	60.31	781.655	61.01	781.643
81.39	780.719	81.95	780.693	87.12	780.377	87.27	780.368	87.66	780.366
101.29	780.292	101.31	780.3	102.15	780.561	102.52	780.695	103.84	781.176
107.05	781.537	108.07	781.651	111.39	782.95	111.51	782.998	114.83	783.535
114.95	783.555	115.07	783.599	118.4	784.794	118.85	784.78	119.89	784.749

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.025	0	.025	118.4	.025

Bank Sta: Left Right Coeff Contr. Expan.  
0 118.4 .1 .3

Downstream Deck/Roadway Coordinates num= 4

Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord
0	788.45	784.68	0	788.45	785.55	150	788.45	785.55	
150	788.45	784.54							

Downstream Bridge Cross Section Data  
Station Elevation Data num= 36

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	786.874	.06	786.874	.14	786.873	1.71	786.277	5.45	784.859
10.05	784.549	10.09	784.546	10.19	784.522	21.37	781.889	24.3	782.157
24.34	782.16	24.39	782.16	28.43	782.186	60.31	781.655	61.01	781.643
81.39	780.719	81.95	780.693	87.12	780.377	87.27	780.368	87.66	780.366
101.29	780.292	101.31	780.3	102.15	780.561	102.52	780.695	103.84	781.176
107.05	781.537	108.07	781.651	111.39	782.95	111.51	782.998	114.83	783.535
114.95	783.555	115.07	783.599	118.4	784.794	118.85	784.78	119.89	784.749

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.025	0	.025	118.4	.025

Bank Sta: Left Right Coeff Contr. Expan.  
0 118.4 .1 .3

Upstream Embankment side slope = 2 horiz. to 1.0 vertical  
Downstream Embankment side slope = 2 horiz. to 1.0 vertical  
Maximum allowable submergence for weir flow = .98  
Elevation at which weir flow begins =  
Energy head used in spillway design =  
Spillway height used in design =  
weir crest shape = Broad Crested

Number of Piers = 3

Pier Data  
Pier Station Upstream= 28.43 Downstream= 28.43  
Upstream num= 4  
width Elev width Elev width Elev width Elev  
2.624 776.118 2.624 778.249 2.624 780.099 2.624 786  
Downstream num= 4  
width Elev width Elev width Elev width Elev  
2.624 776.118 2.624 778.249 2.624 780.099 2.624 786

Pier Data  
Pier Station Upstream= 60.43 Downstream= 60.43  
Upstream num= 4  
width Elev width Elev width Elev width Elev  
2.624 776.118 2.624 778.249 2.624 780.099 2.624 786  
Downstream num= 4  
width Elev width Elev width Elev width Elev  
2.624 776.118 2.624 778.249 2.624 780.099 2.624 786

Pier Data  
Pier Station Upstream= 93.29 Downstream= 93.29  
Upstream num= 4  
width Elev width Elev width Elev width Elev  
2.624 776.118 2.624 777.581 2.624 779.849 2.624 786  
Downstream num= 4

KHRNov2013.rep					
Width	Elev	Width	Elev	Width	Elev
2.624	776.118	2.624	777.581	2.624	779.849
	81.95		81.95		81.95

Number of Bridge Coefficient Sets = 1

Low Flow Methods and Data

Energy  
Selected Low Flow Methods = Highest Energy Answer

High Flow Method  
Energy Only

Additional Bridge Parameters

Add Friction component to Momentum  
Do not add Weight component to Momentum  
Class B flow critical depth computations use critical depth  
inside the bridge at the upstream end  
Criteria to check for pressure flow = Upstream energy grade line

CROSS SECTION

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 392

INPUT

Description: K54b - Oct 2013

Station Elevation Data num= 36					
Sta	Elev	Sta	Elev	Sta	Elev
0	786.874	.06	786.874	.14	786.873
10.05	784.549	10.09	784.546	10.19	784.522
24.34	782.16	24.39	782.16	28.43	782.186
81.39	780.719	81.95	780.693	87.12	780.377
101.29	780.292	101.31	780.3	102.15	780.561
107.05	781.537	108.07	781.651	111.39	782.95
114.95	783.555	115.07	783.599	118.4	784.794
119.96	784.749				

Manning's n Values num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
0	.025	0	.025	118.4	.025

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	0	118.4		117	117	.1	.3

CROSS SECTION

RIVER: Thalweg\_AllSurve  
REACH: Thalweg\_AllSurve RS: 274.99

INPUT

Description: K9 - Oct 2013

Station Elevation Data num= 51					
Sta	Elev	Sta	Elev	Sta	Elev
1.59	786.9	1.93	783.26	1.94	783.261
6.62	781.853	6.92	781.739	7.63	781.784
33.38	782.002	34.44	782.24	35.3	782.433
43.88	782.699	52.26	782.684	54.11	782.553
59.37	782.932	61.58	782.575	61.68	782.56
66.7	782.489	68.41	782.622	69.52	782.622
98.54	782.365	102.51	782.157	102.53	782.156
118.87	780.823	118.92	780.82	118.98	780.82

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KHRNov2013.rep					
Width	Elev	Width	Elev	Width	Elev
148.48	780.897	148.8	780.904	153.86	781.02
158.65	780.611	161.25	781.562	161.31	781.594
166.34	784.177				

Manning's n Values num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
1.59	.025	1.59	.025	166.34	.025

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	1.59	166.34		0	0	.1	.3

SUMMARY OF MANNING'S N VALUES

River: Columbia

Reach	River Sta.	n1	n2	n3
US	2	.022	.022	.022
US	1	.022	.022	.022
Downstream	4	.022	.022	.022
Downstream	3	.022	.022	.022
Downstream	2	.022	.022	.022
Downstream	1.5	Bridge		
Downstream	1	.022	.022	.022
Downstream	0	.022	.022	.022

River: Thalweg\_AllSurve

Reach	River Sta.	n1	n2	n3
Thalweg_AllSurve	3583	.032	.032	.032
Thalweg_AllSurve	3519	.032	.032	.032
Thalweg_AllSurve	3132	.032	.032	.032
Thalweg_AllSurve	2742	.032	.032	.032
Thalweg_AllSurve	2571	.032	.032	.032
Thalweg_AllSurve	2443	.032	.032	.032
Thalweg_AllSurve	2312	.032	.032	.032
Thalweg_AllSurve	2213	.032	.032	.032
Thalweg_AllSurve	2212.8	Bridge		
Thalweg_AllSurve	2184	.032	.032	.032
Thalweg_AllSurve	2068	.032	.032	.032
Thalweg_AllSurve	1971	.025	.025	.025
Thalweg_AllSurve	1679	.025	.025	.025
Thalweg_AllSurve	1483	.025	.025	.025
Thalweg_AllSurve	1239	.025	.025	.025
Thalweg_AllSurve	1106	.025	.025	.025
Thalweg_AllSurve	1015	.025	.025	.025
Thalweg_AllSurve	899.86	.025	.025	.025
Thalweg_AllSurve	772	.025	.025	.025
Thalweg_AllSurve	643.15	.025	.025	.025
Thalweg_AllSurve	463.66	.025	.025	.025
Thalweg_AllSurve	410.07	.025	.025	.025
Thalweg_AllSurve	409	Bridge		
Thalweg_AllSurve	392	.025	.025	.025
Thalweg_AllSurve	274.99	.025	.025	.025

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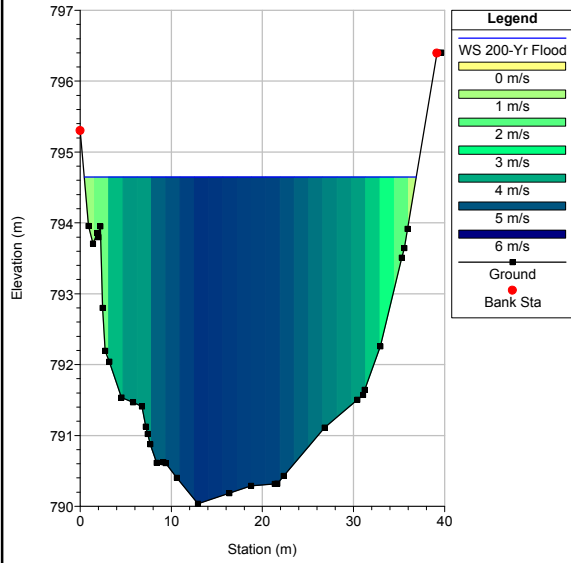
**2013 HEC-RAS Results Summary Table**

200 year Flood Results														
Section Name	Stationing	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	Vel Chnl	Flow Area	Top Width	Froude # Chl	Left Bank	Left FB	Right Bank	Right FB
	m		(m3/s)	(m)	(m)	(m)	(m/s)	(m2)	(m)		elev (m)	(m)	elev (m)	(m)
K1	3583	200-yr	570	790.04	794.65		4.64	122.72	36.42	0.81	795.3	0.65	796.82	2.17
K52	3519	200-yr	570	791.25	794.53		4.02	141.78	53.69	0.79	795.64	1.11	796.64	2.11
K2	3132	200-yr	570	789.54	792.64	792.34	3.89	146.39	64.22	0.82	793.84	1.20	795.21	2.57
K50	2742	200-yr	570	787.63	790.52		3.21	177.71	121.02	0.84	791.6	1.08	792.3	1.78
K60	2571	200-yr	570	786.48	790.23		2.18	260.95	110.69	0.45	790.93	0.70	792	1.77
K51	2443	200-yr	570	785.34	789.14		4.29	132.84	47.63	0.82	790.64	1.50	789.85	0.71
K4	2312	200-yr	570	785.28	788.93		3.28	173.67	61.67	0.62	789.81	0.88	789.8	0.87
K10	2213	200-yr	570	784.25	788.80	787.49	2.48	230.1	89.95	0.49	790.3	1.50	790.32	1.52
Highway 95 Bridge														
K11	2184	200-yr	570	784.02	788.66		2.68	212.98	112.31	0.49	790.49	1.83	790.49	1.83
K11B	2068	200-yr	570	784.09	787.81		4.02	141.83	61.69	0.85	788.79	0.98	788.18	0.37
K5	1971	200-yr	570	783.78	787.66		3.4	167.82	64.33	0.67	788.5	0.84	788.6	0.94
K61	1679	200-yr	570	783.28	786.79		3.86	147.61	55.22	0.75	787.35	0.56	787.89	1.10
K6	1483	200-yr	570	783.17	786.00	785.79	4.27	133.35	56.29	0.89	786.6	0.60	787.82	1.82
K6A	1239	200-yr	570	782.15	785.34		3.46	164.77	91.78	0.82	786.33	0.99	786.8	1.46
K7	1106	200-yr	570	781.09	785.15		2.83	201.38	94.28	0.62	786.75	1.60	786.94	1.79
K7A	1015	200-yr	570	780.86	784.84		3.17	179.69	85.71	0.7	786.03	1.19	786.29	1.45
K62	900	200-yr	570	780.79	784.75		2.6	219	95.27	0.55	785.22	0.47	785.8	1.05
K7B	772	200-yr	570	780.57	784.63		2.26	252.52	129.12	0.52	784.97	0.34	785.31	0.68
K8	643	200-yr	570	780.51	784.48		2.24	254.75	108.17	0.47	785.09	0.61	785.25	0.77
K53	464	200-yr	570	780.55	783.96		3.14	181.7	76.45	0.65	787.12	3.16	785.14	1.18
K54	410	200-yr	570	780.29	784.06	782.98	2.24	254.51	104.24	0.46	787.12	3.06	786.59	2.53
CPR Bridge														
K9	275	200-yr	570	780.61	783.98		1.72	332.03	164.06	0.39	786.9	2.92	784.76	0.78



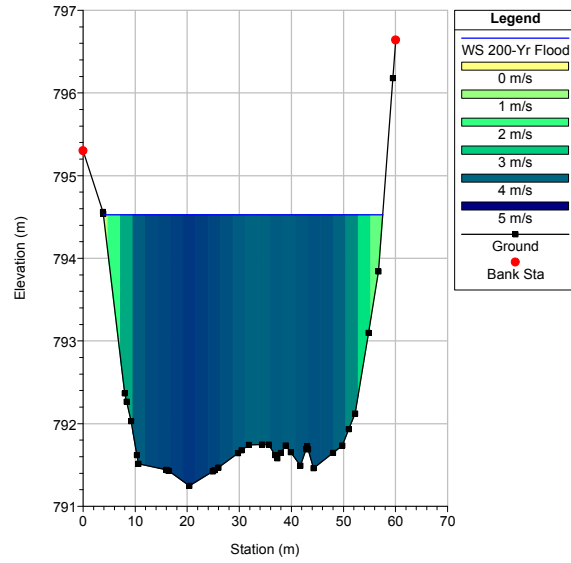
KHR Nov2013 Plan: Plan 30 1/22/2014

River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 3583 X-Section K1 - Oct 2013



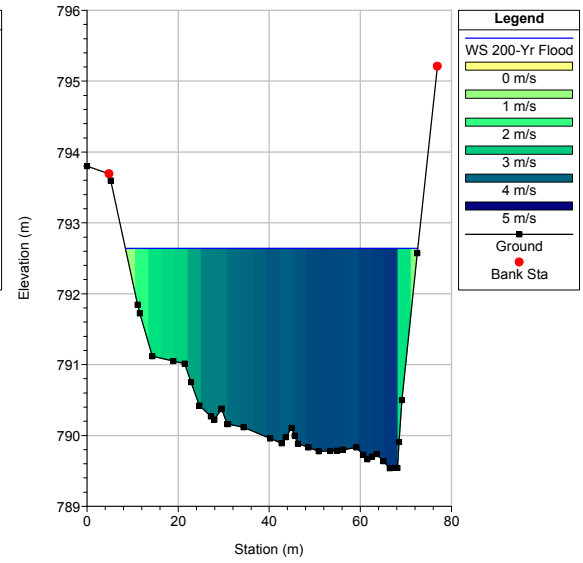
KHR Nov2013 Plan: Plan 30 1/22/2014

River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 3519 X-Section K52 - Oct 2013



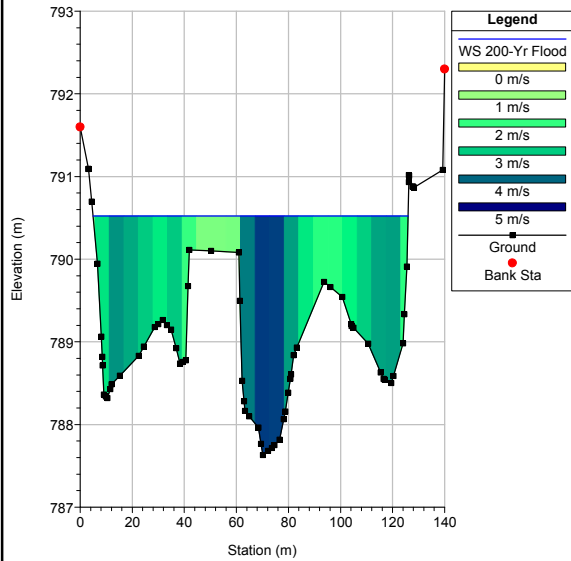
KHR Nov2013 Plan: Plan 30 1/22/2014

River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 3132 Section K2 - Oct 2013



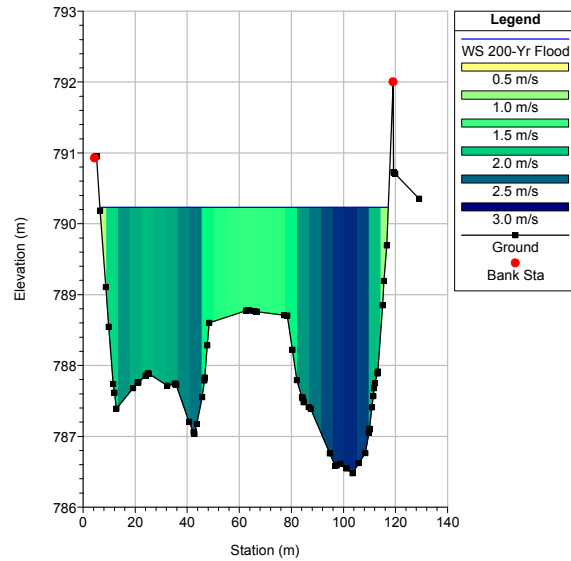
KHR Nov2013 Plan: Plan 30 1/22/2014

River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 2742 K50 - Oct 2013



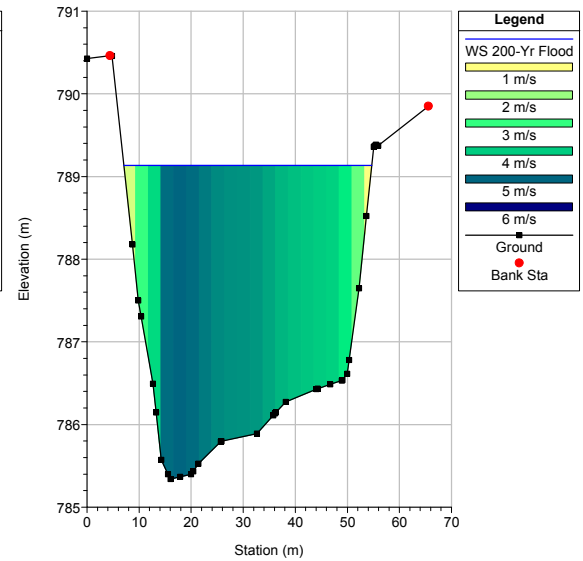
KHR Nov2013 Plan: Plan 30 1/22/2014

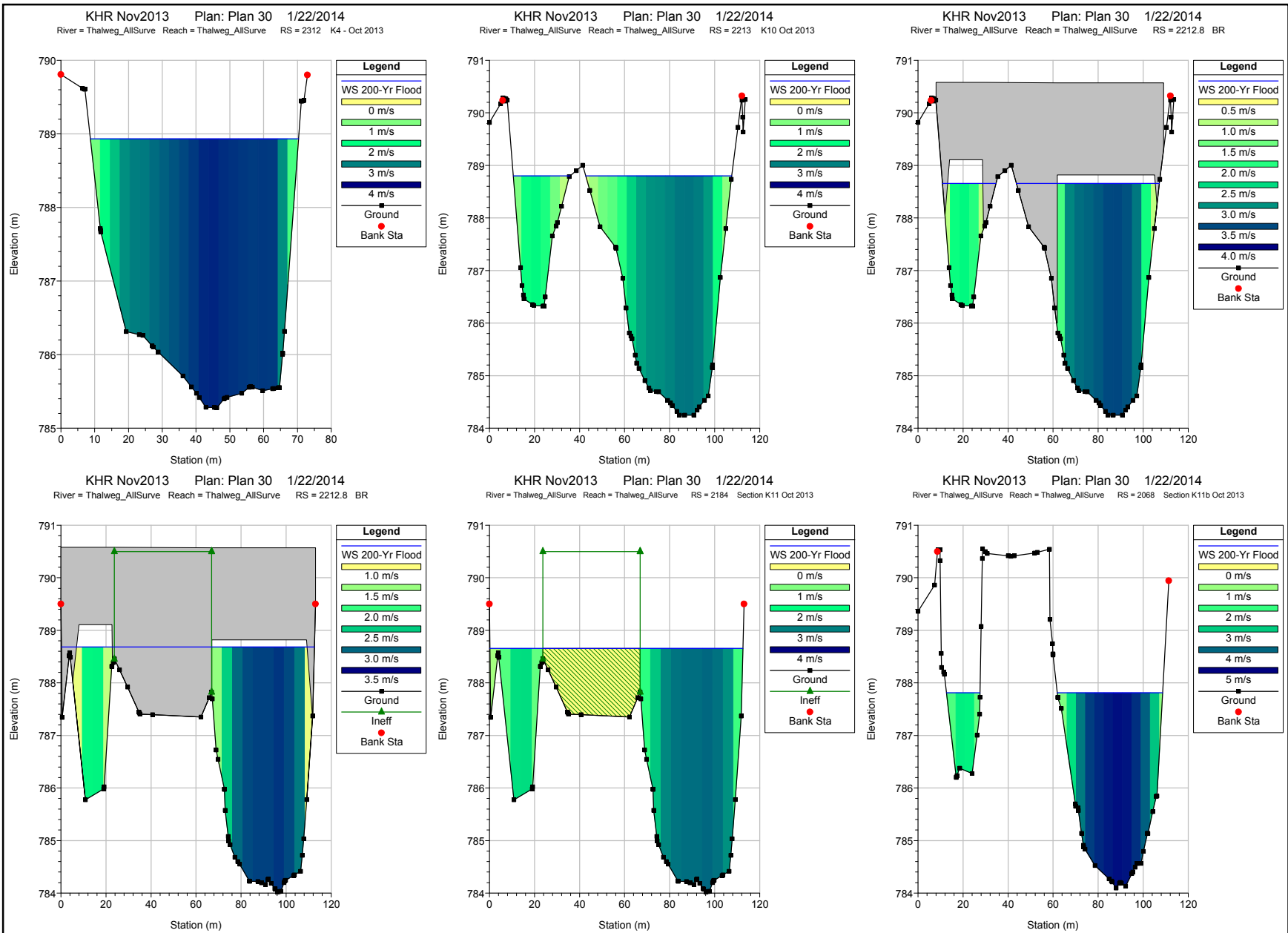
River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 2571 K60 - Oct 2013



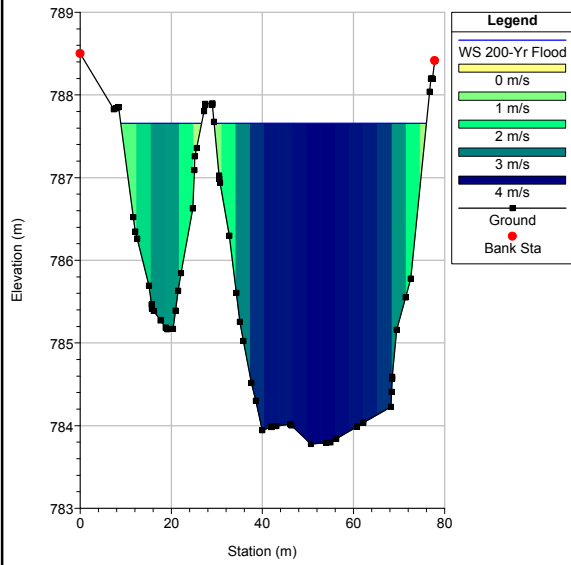
KHR Nov2013 Plan: Plan 30 1/22/2014

River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 2443 K51 - Oct 2013

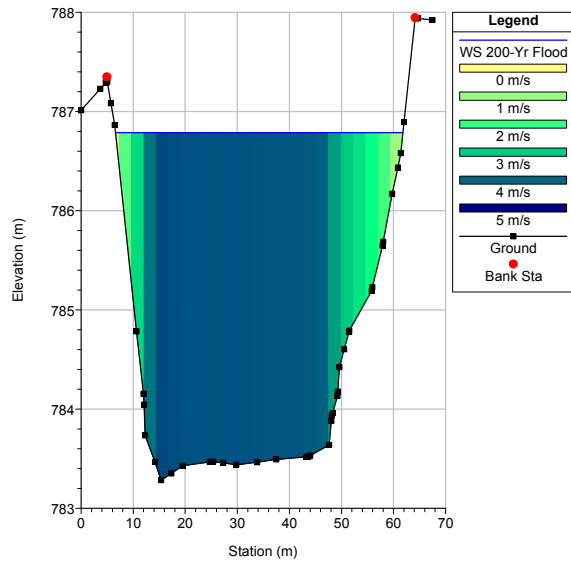




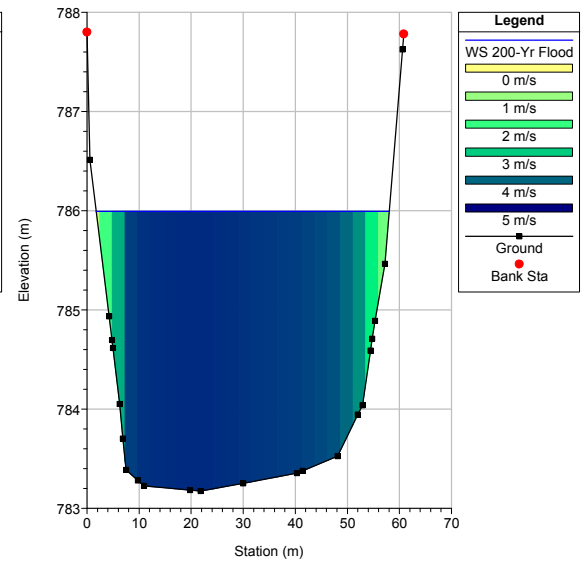
KHR Nov2013 Plan: Plan 30 1/22/2014  
River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 1971 K5 - Oct 2013



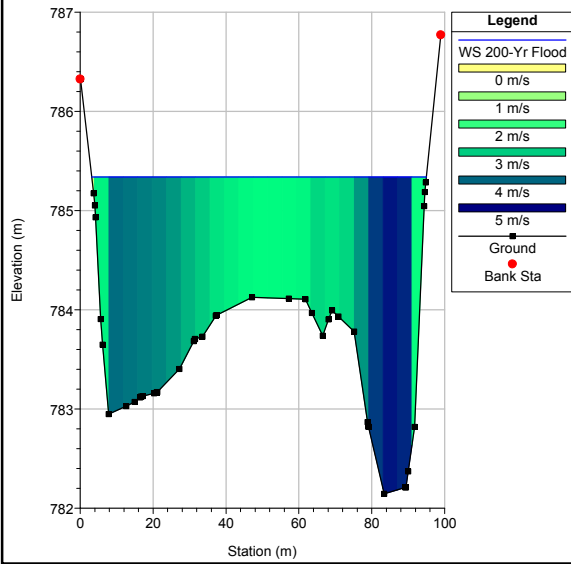
KHR Nov2013 Plan: Plan 30 1/22/2014  
River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 1679 K61 - Oct 2013



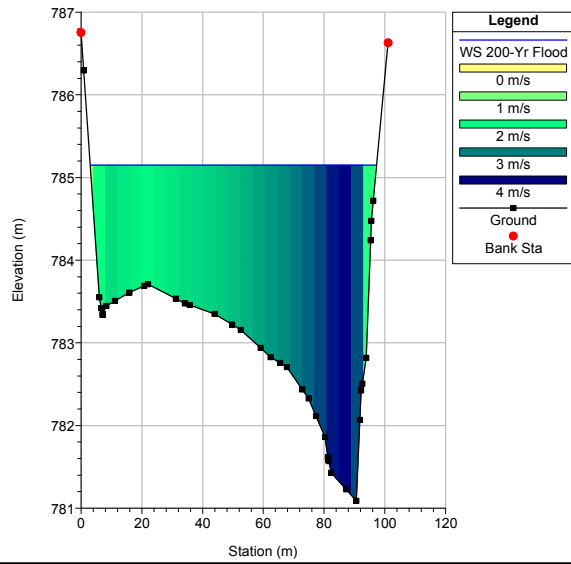
KHR Nov2013 Plan: Plan 30 1/22/2014  
River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 1483 K6 - Oct 2013



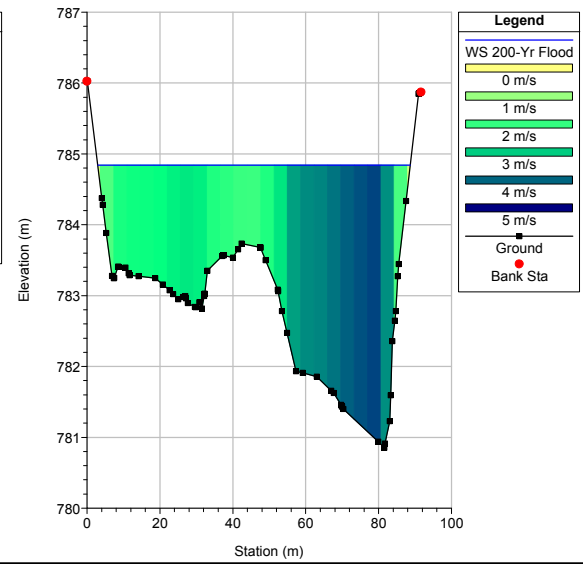
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River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 1239 K6A - Oct 2013



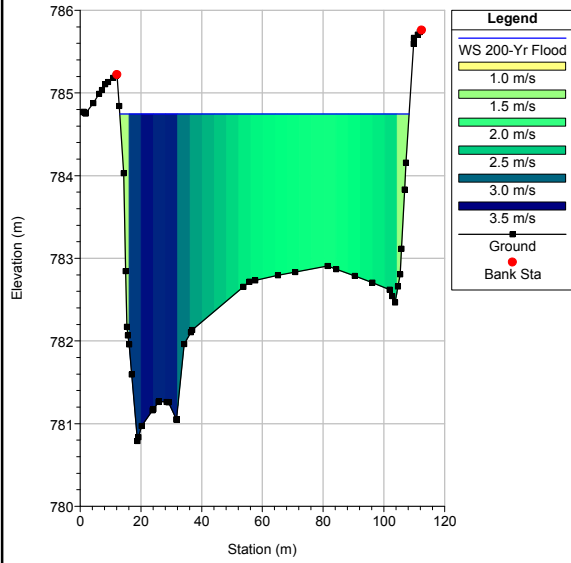
KHR Nov2013 Plan: Plan 30 1/22/2014  
River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 1106 K7 - Oct 2013



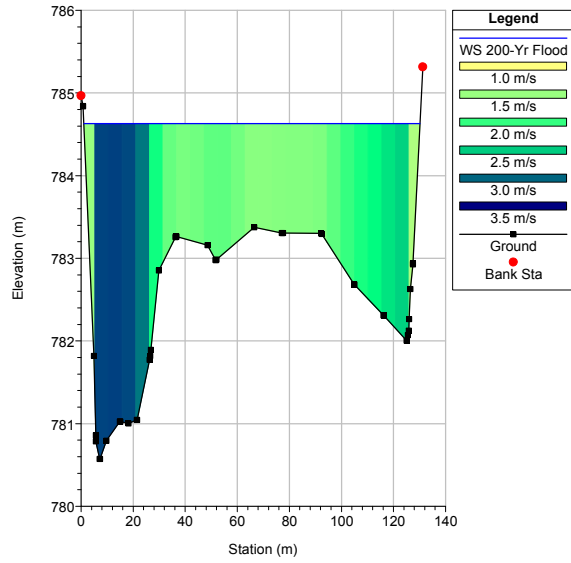
KHR Nov2013 Plan: Plan 30 1/22/2014  
River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 1015 K7A - Oct 2013



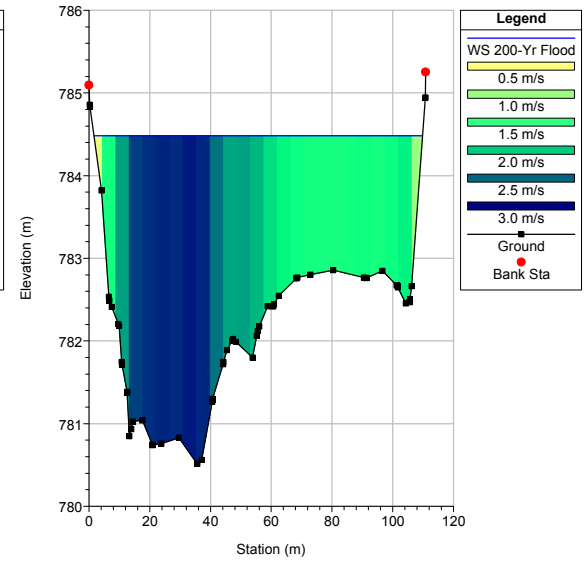
KHR Nov2013 Plan: Plan 30 1/22/2014  
 River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 899.86 K62 - Oct 2013



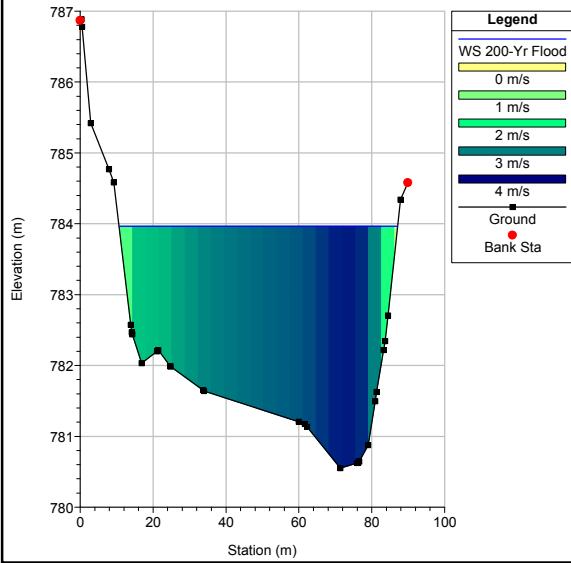
KHR Nov2013 Plan: Plan 30 1/22/2014  
 River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 772 K7B - Oct 2013



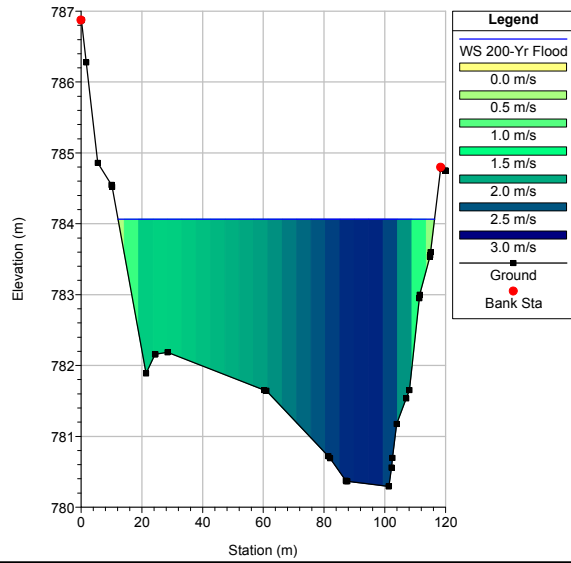
KHR Nov2013 Plan: Plan 30 1/22/2014  
 River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 643.15 K8 - Oct 2013



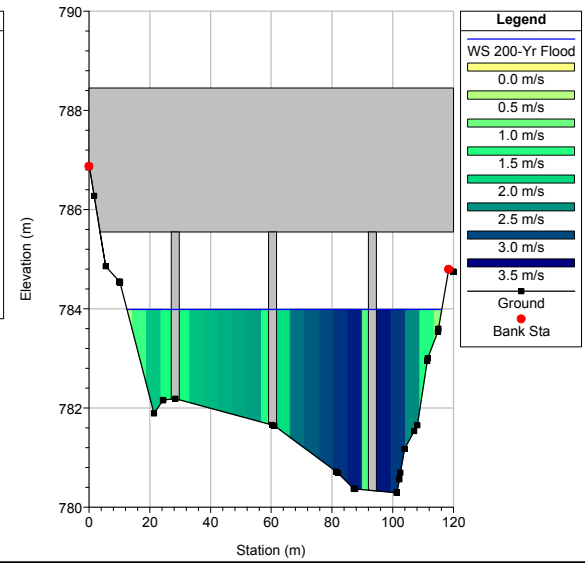
KHR Nov2013 Plan: Plan 30 1/22/2014  
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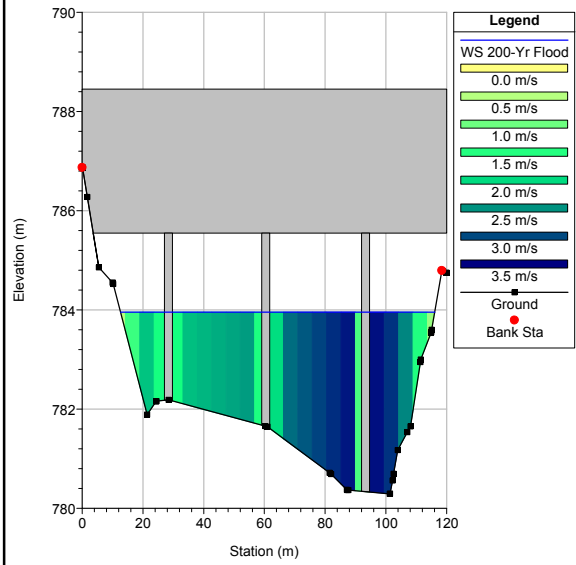
KHR Nov2013 Plan: Plan 30 1/22/2014  
 River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 410.07 K54 - Oct 2013



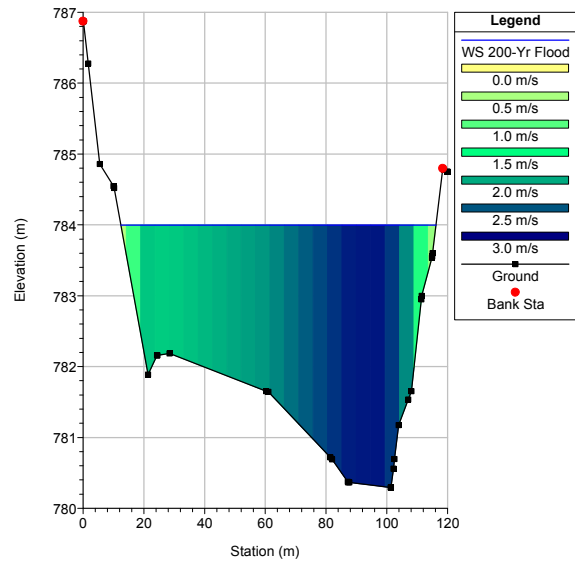
KHR Nov2013 Plan: Plan 30 1/22/2014  
 River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 409 BR



KHR Nov2013 Plan: Plan 30 1/22/2014  
 River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 409 BR



KHR Nov2013 Plan: Plan 30 1/22/2014  
 River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 392 K54b - Oct 2013



KHR Nov2013 Plan: Plan 30 1/22/2014  
 River = Thalweg\_AllSurve Reach = Thalweg\_AllSurve RS = 274.99 K9 - Oct 2013

